



**OFFICE OF STATEWIDE HEALTH PLANNING AND DEVELOPMENT
FACILITIES DEVELOPMENT DIVISION**

**APPLICATION FOR OSHPD PREAPPROVAL
OF MANUFACTURER'S CERTIFICATION (OPM)**

OFFICE USE ONLY

APPLICATION #: OPM-0064-13

OSHPD Preapproval of Manufacturer's Certification (OPM)

Type: New Renewal Update to Pre-CBC 2013 OPA Number: _____

Manufacturer Information

Manufacturer: The Baker Company

Manufacturer's Technical Representative: Larry McCarthy

Mailing Address: P.O. Drawer E, Sanford, Maine 04073

Telephone: (207) 324-8773 ext.374 Email: lmccarthy@bakerco.com

Product Information

Product Name: SterilGARD

Product Type: Class II, Type A2, Biosafety Cabinet

Product Model Number: SG404, SG504, & SG604

General Description: Biological Safety Cabinets, SterilGARD Models, SG404, SG504, & SG604, floor mounted and wall mounted anchorage.

Applicant Information

Applicant Company Name: The Baker Company

Contact Person: Larry McCarthy

Mailing Address: P.O. Drawer E, Sanford, Maine 04073

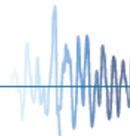
Telephone: (207) 324-8773 ext.374 Email: lmccarthy@bakerco.com

I hereby agree to reimburse the Office of Statewide Health Planning and Development review fees in accordance with the California Administrative Code, 2013.

Signature of Applicant: *Larry McCarthy* Date: 10/3/2013

Title: Product Engineer Company Name: The Baker Company

"Access to Safe, Quality Healthcare Environments that Meet California's Diverse and Dynamic Needs"





**OFFICE OF STATEWIDE HEALTH PLANNING AND DEVELOPMENT
FACILITIES DEVELOPMENT DIVISION**

Registered Design Professional Preparing Engineering Recommendations

Company Name: Brandow and Johnston, Inc.

Name: Peter Maranian California License Number: SE 2720

Mailing Address: 700 South Flower Street, Suite 1800, Los Angeles, CA 90017

Telephone: (213) 596-4575 Email: pmaranian@bjsce.com

OSHPD Special Seismic Certification Preapproval (OSP)

- Special Seismic Certification is preapproved under OSP- (Separate application for OSP is required)
- Special Seismic Certification is no preapproved

Certification Method(s)

- Testing in accordance with: ICC-ES AC156 FM 1950-10
- Other* (Please Specify): _____

Testing was performed in accordance with ICC-ES AC156 (dated 12/17 & 18/2007)

*Use of criteria other than those adopted by the California Building Standards Code, 2013 (CBSC 2013) for component supports and attachments are not permitted. For distribution system, interior partition wall, and suspended ceiling seismic bracings, test criteria other than those adopted in the CBSC 2013 may be used when approved by OSHPD prior to testing.

- Analysis
- Experience Data
- Combination of Testing, Analysis, and/or Experience Data (Please Specify): _____

List of Attachments Supporting the Manufacturer's Certification

- Test Report Drawings Calculations Manufacturer's Catalog
- Other(s) (Please Specify): _____

OFFICE USE ONLY – OSHPD APPROVAL VALID FOR CBC 2013 ONLY

Signature:  Date: April 23, 2014

Print Name: Jeffrey Y. Kikumoto

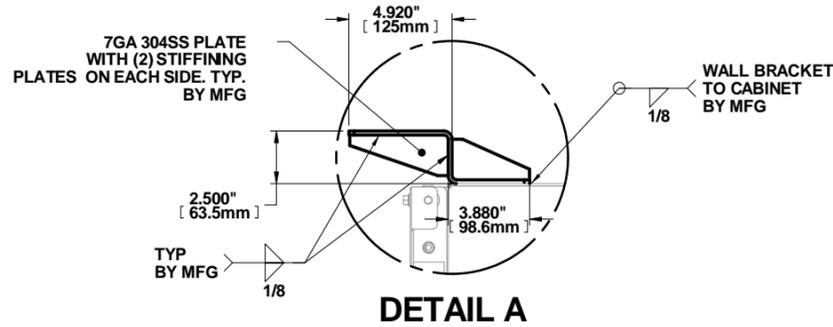
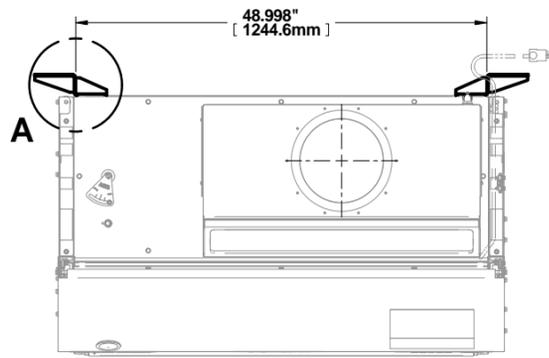
Title: Senior Structural Engineer

Condition of Approval (if applicable): _____

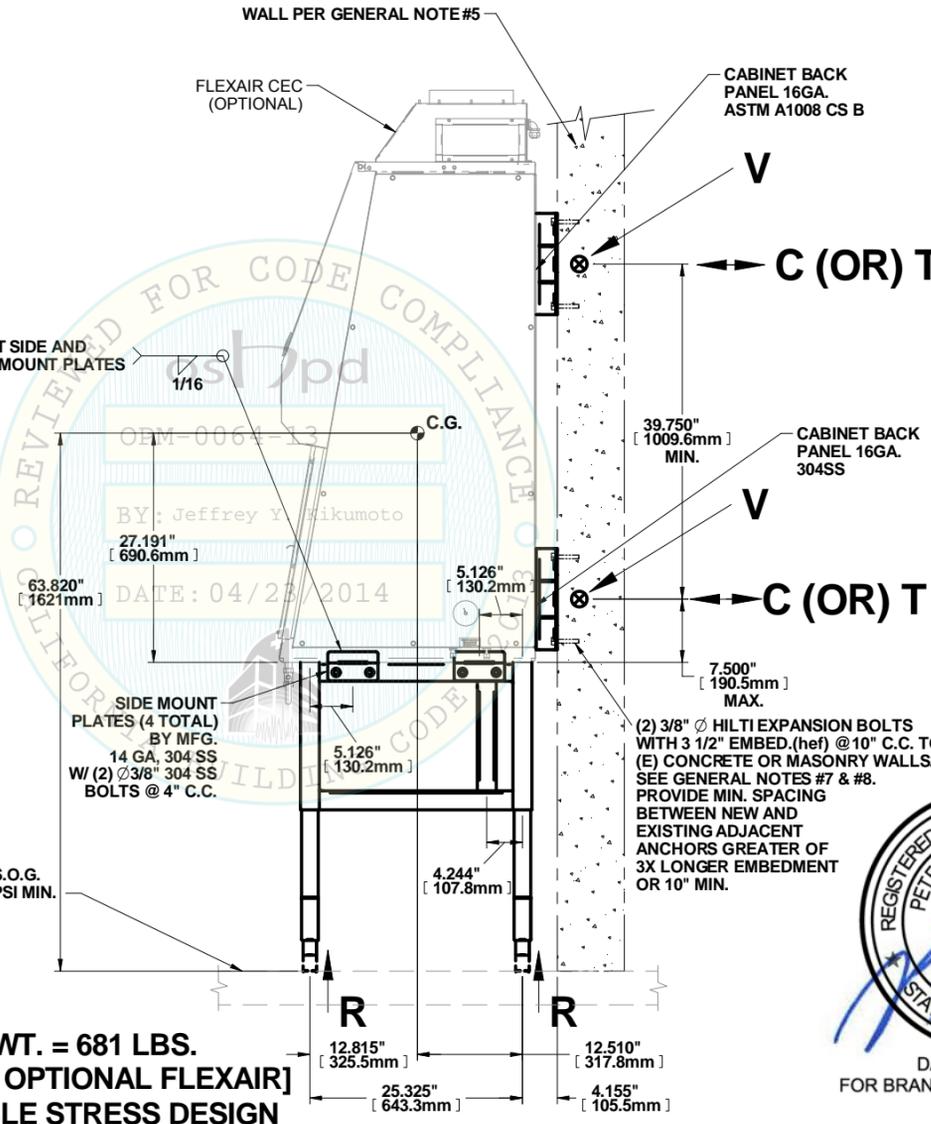
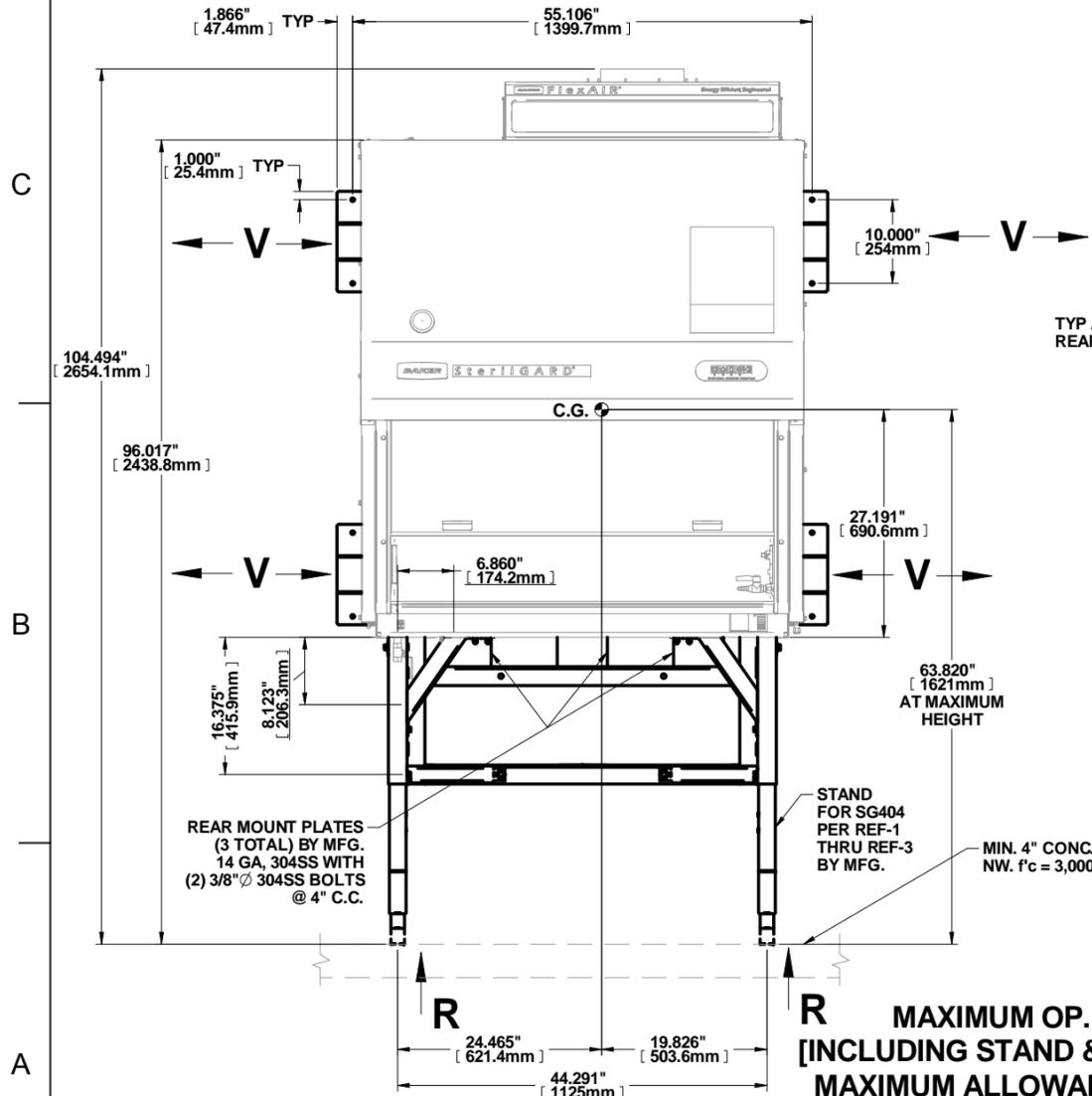
"Access to Safe, Quality Healthcare Environments that Meet California's Diverse and Dynamic Needs"



REV	DESCRIPTION	DATE	DRN	APP BY	APP DATE
-	SEE REV F FOR ALL PAST REVISIONS LISTED	-	-	-	-
G	UPDATED DRAWING PER PE COMMENTS	4/22/2014	LAM	-	-



- GENERAL NOTES:**
- THIS OSHPD PREAPPROVAL OF MANUFACTURER'S CERTIFICATION (OPM) IS BASED ON CBC 2013. THE DEMAND (DESIGN FORCES) FOR USE WITH THIS OPM SHALL BE BASED ON THE CBC 2013.
 - DESIGN SEISMIC ANCHORAGE FORCES PER ASCE 7-10 SECTION 13.3.1, EQUATIONS 13.3-1, 13.3-2, AND 13.3-3 WHERE $S_{DS} \leq 2.0$, $z/h \leq 1.0$, $a_p = 1.0$, $I_p = 1.5$, AND $R_p = 2.5$. $F_p W_p / 1.4 \leq 1.03$ AND $O_0 = 2.5$.
 - THIS PRE-APPROVAL IS APPLICABLE TO ANY LOCATION IN THE STATE OF CALIFORNIA, AT ANY HEIGHT WITHIN THE BUILDING WHERE $S_{DS} \leq 2.0$.
 - THIS PRE-APPROVAL COVERS ONLY THE SUPPORT & ATTACHMENTS OF THE UNIT TO THE HOSPITAL BUILDING'S STRUCTURE.
 - MAXIMUM REACTIONS TO THE STRUCTURE SHOWN ON THE DRAWINGS ARE ALLOWABLE STRESS DESIGN LOADS. SEE NOTES BELOW FOR SEOR'S RESPONSIBILITIES.
 - THE CONCRETE WALL SHALL BE MIN. 8" THICK $f'_c = 3,000$ PSI MIN. CONC. N.W. OR SAND L.W.. THE MASONRY WALL SHALL BE MIN. 8" THICK $f_m = 1,500$ PSI MIN. FULLY GROUTED.
 - ANCHOR BOLTS INTO (E) CONCRETE WALL TO BE HILTI KB-TZ (ICC ESR-1917). PERIODIC INSPECTION REQUIRED PER CBC 2013 SECTION 1705A.3. REFER TO ICC ESR REPORT FOR INSTALLATION REQUIREMENTS. TESTING OF EXPANSION ANCHORS PER 2013 CBC, 1913A.7 AND NOTE# 11 BELOW.
 - ANCHOR BOLTS INTO (E) GROUT FILLED MASONRY WALL TO BE HILTI KB3 (ICC ESR-1385). CONTINUOUS INSPECTION REQUIRED PER CBC 2013 SECTION 1705A.4. REFER TO ICC ESR REPORT FOR INSTALLATION REQUIREMENTS. TESTING OF EXPANSION ANCHORS PER 2013 CBC, 1913A.7. SEE NOTE# 16 FOR SEOR RESPONSIBILITIES.
 - MAINTAIN 12" CLEARANCE FROM ANY EDGES OF (E) CONCRETE OR MASONRY WALL.
 - LOCATE (E) REINFORCEMENTS BEFORE DRILLING INTO (E) CONCRETE OR MASONRY WALL. DO NOT CUT ANY (E) REINFORCEMENTS.
 - TORQUE TEST:
 - TEST INSTALLED ANCHORS AT A RATE OF 50% IN THE PRESENCE OF INSPECTOR OF RECORD. (I.O.R.) A REPORT OF THE TEST RESULTS SHALL BE SUBMITTED TO OSHPD.
 - ALL WEDGE ANCHORS (LOADED IN EITHER PULLOUT OR SHEAR) SHALL BE TORQUE TESTED WITH CALIBRATED TORQUE WRENCH TO 25 FT-LBS FOR HILTI KB-TZ IN CONCRETE & 15 FT-LBS FOR HILTI KB3 IN MASONRY. THE SPECIFIED TORQUE SHALL BE ATTAINED WITHIN 1/2 TURN OF THE NUT. THE SPECIFIED TORQUE IS THE INSTALLATION VERIFICATION TEST LOAD. TORQUED BOLTS SHALL BE TESTED A MINIMUM OF 24 HOURS AFTER INSTALLATION AND SHALL NOT HAVE MORE THAN 1/2 TURN OF THE NUT.
 - WHERE ANCHORS FAIL TESTING, REPLACE DEFICIENT ANCHORS AND RETEST. TEST ALL ANCHORS OF THE SAME TYPE INSTALLED BY THE SAME TRADE AND NOT PREVIOUSLY TESTED.
 - WEDGE ANCHOR EMBEDMENT IS EQUAL TO EFFECTIVE MIN. EMBEDMENT (hef) AS DEFINED IN ICC-ESR REPORTS.
 - SEOR TO VERIFY THAT THE INSTALLATION IS IN CONFORMANCE WITH THE 2013 CBC AND WITH THE NOTES AND DETAILS SHOWN IN THIS PRE-APPROVAL. VERIFY THAT THE EQUIPMENT'S ACTUAL WEIGHT, CG LOCATION, ANCHOR LOCATIONS, ANCHOR DETAILS AND THE MATERIAL AND GAGE OF THE UNIT WHERE ATTACHMENTS ARE MADE CONFORM TO THE INFORMATION SHOWN IN THIS PRE-APPROVAL. BLDG. SEOR TO VERIFY THAT PROJECT SPECIFIC VALUES OF S_{DS} & z/h RESULT IN SEISMIC FORCES (E_n , E_v) THAT DO NOT EXCEED THE VALUES ON THE DETAILS.
 - THE BUILDING SEOR TO VERIFY THAT THE (N) OR (E) STRUCTURE IS ADEQUATE FOR THE DESIGN LOADS IMPOSED ON IT BY THIS EQUIPMENT IN ADDITION TO ALL OTHER LOADS. PROVIDE & DESIGN SUPPLEMENTARY FRAMING MEMBERS AS REQ'D. REFER TO MAXIMUM REACTIONS TO THE STRUCTURE.
 - SEOR SHALL VERIFY THAT:
 - MASONRY IS NOT CRACKED AS DEFINED IN ICC-ES AC01 SECTION 2.3. SEOR SHALL PROVIDE CALCULATIONS TO SHOW THAT THE MASONRY WALL WOULD NOT CRACK UNDER THE DESIGN EARTHQUAKE LOADS UNDER ALL SERVICE LOAD CONDITIONS; THE WALL HAS TO REMAIN ELASTIC.
 - MASONRY IS FULLY GROUTED IN ACCORDANCE W/ ESR-1385 SECTION 3.2.
 - CONDITION OF USE REQUIREMENTS IN ACCORDANCE W/ ESR-1385 SECTION 5.0 IS SATISFIED.
 - RESPONSIBILITIES OF THE SEOR OF THE BLDG.: THE NEW ANCHORS ARE LOCATED AT AN ADEQUATE DISTANCE FROM ANY NEW OR EXISTING ANCHORS. THE SPACING SHOWN IN DETAILS ARE A MINIMUM FOR THE BOLT SIZE SHOWN. THE REQ'D SPACINGS FROM OTHER ANCHORS OF OTHER DIAMETERS & EMBEDMENTS MAY VARY & SHALL BE EVALUATED BY THE SEOR.
 - BLDG. SEOR TO VERIFY THE CONCRETE MEETS THE REQUIREMENTS OF THE APPLICABLE ICC-ESR.



DATE: 04/23/14
FOR BRANDOW & JOHNSTON, INC

OPM-0064-13

NOTE:
THE WALL SEISMIC RESTRAINTS HAVE PASSED TESTING USING AC156 ACCEPTANCE CRITERIA FOR SEISMIC QUALIFICATION BY SHAKE TABLE TESTING OF NON-STRUCTURAL COMPONENTS AND SYSTEMS.

FRONT VIEW

**MAXIMUM OP. WT. = 681 LBS.
[INCLUDING STAND & OPTIONAL FLEXAIR]
MAXIMUM ALLOWABLE STRESS DESIGN
REACTIONS TO THE STRUCTURE
[LBS./POST]**

SLAB LOC'N	$F_p/W_p/1.4$	V	C	T	R
ELEV. SLAB	1.03	180	190	190	250
ON GRADE	0.64	110	120	120	250

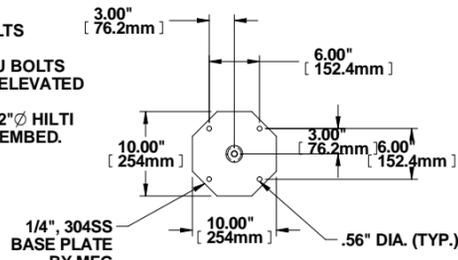
SIDE VIEW

TABULATED REACTIONS DO NOT REFLECT THE APPLICATION OF THE OVERSTRENGTH FACTOR (Ω_0) TO ACCOUNT FOR ANCHORAGE TO CONCRETE.

UNLESS OTHERWISE SPECIFIED DO NOT SCALE DRAWING U.S. CUSTOMARY TOLERANCES (INCHES) REMOVE ALL BURRS AND SHARP EDGES		THIS DRAWING AND ITS CONTENTS ARE THE PROPERTY OF THE BAKER COMPANY. THE INFORMATION CONTAINED HEREON MAY NOT BE USED OR COPIED IN ANY MANNER WITHOUT THE WRITTEN PERMISSION OF THE BAKER COMPANY.	
DECIMALS	MACHINED	SHEET METAL	ANGULAR
X	+0.02	+0.06	±30' (1:7)
XX	+0.01	+0.03	THIRD ANGLE
XXX	+0.005	+0.010	PROJECTION
FRACTIONS	+1/32	NO 4	
FINISH	✓		
DATE	10/2/2013	SIZE	D
DRAWN	L.McCarthy	DWG NO	BGA-SG404-A2-1-4-111R
CHECKED		DATE	
			2 OF 2

REV	DESCRIPTION	DATE	DRN	APP BY	APP DATE
-	SEE REV. F FOR ALL PAST REVISIONS LISTED	-	-	-	-
G	UPDATED DRAWING PER PE COMMENTS	4/22/2014	LAM	-	-

- NOTE:**
1. INSTALL 2 BOLTS DIAGONALLY.
 2. PROVIDE THRU BOLTS PER "B-B" ON ELEVATED FLOOR SLAB.
 3. PROVIDE (2) 1/2" HILTI KB-TZ X 3 1/4" EMBED. (hef) AT S.O.G.



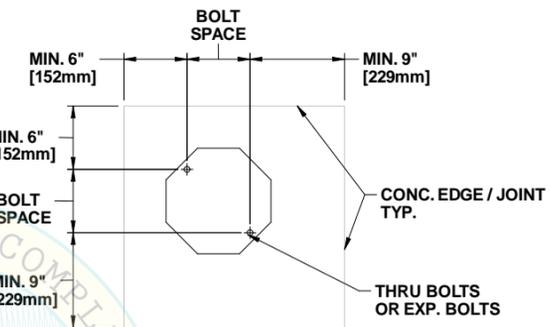
**SECTION "A-A"
BASE PLATE DETAIL**

GENERAL NOTES:

1. THIS OSHPD PREAPPROVAL OF MANUFACTURER'S CERTIFICATION (OPM) IS BASED ON CBC 2013. THE DEMAND (DESIGN FORCES) FOR USE WITH THIS OPM SHALL BE BASED ON THE CBC 2013.
2. DESIGN SEISMIC FORCES PER ASCE 7-10 SECTION 13.3.1, EQUATIONS 13.3-1, 13.3-2, AND 13.3-3 WHERE $S_{DS} \leq 2.0$, $z/h \leq 1.0$, $a_p = 1.0$, $I_p = 1.5$, AND $R_p = 2.5$. $F_p/W_p/1.4 \leq 1.03$ AND $\Omega_0 = 2.5$.
3. THIS PRE-APPROVAL IS APPLICABLE TO ANY LOCATION IN THE STATE OF CALIFORNIA AT ANY HEIGHT WITHIN THE BUILDING.
4. THIS PRE-APPROVAL COVERS ONLY THE SUPPORT & ATTACHMENTS OF THE UNIT TO THE HOSPITAL BUILDING'S STRUCTURE.
5. MAX. REACTIONS TO THE STRUCTURE SHOWN ON THE DRAWINGS ARE ALLOWABLE STRESS DESIGN LOADS. SEE NOTES BELOW FOR SEOR'S RESPONSIBILITIES.
6. THE ELEVATED FLOOR SLAB SHALL BE MIN. 5" THICK, 3000 PSI MIN. CONCRETE SOLID SLAB N.W. OR SAND L.W. CONCRETE FILL ABOVE METAL DECK SHALL BE MIN. 3 1/4" THICK, 3,000 PSI MIN. CONCRETE N.W. OR SAND L.W. THE SLAB-ON-GRADE SHALL BE MIN. 6" THICK, NW, 3,000 PSI MIN.
7. ANCHOR BOLTS INTO (E) CONCRETE TO BE HILTI KB-TZ (ICC ESR-1917). PERIODIC INSPECTION REQUIRED PER CBC 2013 SECTION 1705A.3. REFER TO ICC ESR REPORT FOR INSTALLATION REQUIREMENTS. TESTING OF EXPANSION ANCHORS PER 2013 CBC, 1913A.7 AND NOTE# 11 BELOW.
8. BOLTS THROUGH CONCRETE ON METAL DECK OR ELEVATED SLAB SECTION:
 - A) BOLTS SHALL BE TORQUED BY 1/4 TURN OF THE NUT AFTER SNUG TIGHT (THE SNUG TIGHT CONDITION IS DEFINED AS THE TIGHTNESS REQUIRED TO BRING THE CONNECTED PLIES INTO FIRM CONTACT) CONDITION IS ACHIEVED, UNLESS NOTED OTHERWISE.
 - B) HOLES FOR THROUGH BOLTS SHALL BE 1/16" LARGER THAN BOLT SIZE (HOLE SIZE = BOLT SIZE + 1/16").
 - C) THROUGH BOLTS IN CONCRETE SHALL RECEIVE SPECIAL INSPECTION & TESTING (THROUGH BOLTS WITH STEEL-TO-STEEL CONNECTION IN TENSION DO NOT REQUIRE TESTING) IN ACCORDANCE WITH REQUIREMENTS FOR POST-INSTALLED ANCHORS.
9. MAINTAIN CLEARANCE FROM ANY EDGES OF (E) CONCRETE AS SHOWN
10. LOCATE (E) REINFORCEMENTS BEFORE DRILLING INTO (E) CONCRETE SLAB. DO NOT CUT ANY (E) REINFORCEMENTS.
11. TORQUE TEST:
 - A. TEST INSTALLED ANCHORS AT A RATE OF 50% IN THE PRESENCE OF INSPECTOR OF RECORD, (I.O.R). A REPORT OF THE TEST RESULTS SHALL BE SUBMITTED TO OSHPD.
 - B. ALL WEDGE ANCHORS (LOADED IN EITHER PULLOUT OR SHEAR) SHALL BE TORQUE-TESTED WITH CALIBRATED TORQUE WRENCH TO 25 FT-LBS FOR 3/8" & 40 FT-LBS FOR 1/2" HILTI KB-TZ IN CONCRETE. THE SPECIFIED TORQUE SHALL BE ATTAINED WITHIN 1/2 TURN OF THE NUT. THE SPECIFIED TORQUE IS THE INSTALLATION VERIFICATION TEST LOAD. TORQUED BOLTS SHALL BE TESTED A MINIMUM OF 24 HOURS AFTER INSTALLATION AND SHALL NOT HAVE MORE THAN 1/2 TURN OF THE NUT.
 - C. WHERE ANCHORS FAIL TESTING, REPLACE DEFICIENT ANCHORS AND RETEST. TEST ALL ANCHORS OF THE SAME TYPE INSTALLED BY THE SAME TRADE AND NOT PREVIOUSLY TESTED.
12. WEDGE ANCHOR EMBEDMENT IS EQUAL TO EFFECTIVE MIN. EMBEDMENT (hef) AS DEFINED IN ICC-ESR REPORTS.
13. SEOR TO VERIFY THAT THE INSTALLATION IS IN CONFORMANCE WITH THE 2013 CBC AND WITH THE NOTES AND DETAILS SHOWN IN THIS PRE-APPROVAL. VERIFY THAT THE EQUIPMENT'S ACTUAL WEIGHT, CG LOCATION, ANCHOR LOCATIONS, ANCHOR DETAILS AND THE MATERIAL AND GAGE OF THE UNIT WHERE ATTACHMENTS ARE MADE CONFORM TO THE INFORMATION SHOWN IN THIS PRE-APPROVAL.
14. THE BUILDING SEOR TO VERIFY THAT THE (N) OR (E) STRUCTURE IS ADEQUATE FOR THE DESIGN LOADS IMPOSED ON IT BY THIS EQUIPMENT IN ADDITION TO ALL OTHER LOADS. PROVIDE AND DESIGN SUPPLEMENTARY FRAMING MEMBERS AS REQ'D. REFER TO MAX. REACTIONS TO THE STRUCTURE.
15. RESPONSIBILITIES OF THE SEOR OR THE BLDG.:

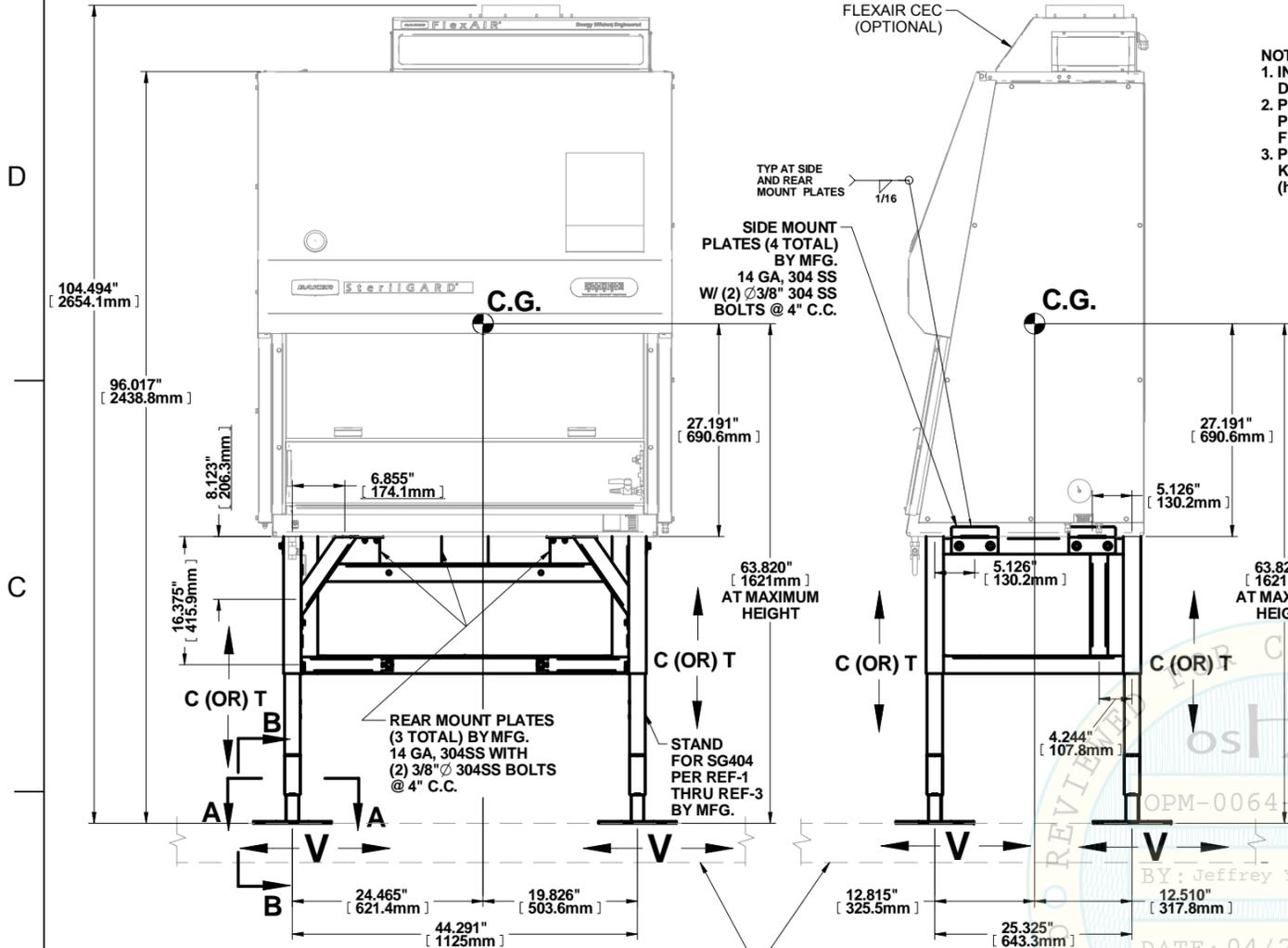
THE NEW ANCHORS ARE LOCATED AT AN ADEQUATE DISTANCE FROM ANY NEW OR EXISTING ANCHORS. THE SPACING SHOWN IN DETAILS ARE A MINIMUM FOR THE BOLT SIZE SHOWN. THE REQUIRED SPACINGS FROM OTHER ANCHORS OF OTHER DIAMETERS & EMBEDMENTS MAY VARY & SHALL BE EVALUATED BY THE SEOR. BLDG. SEOR TO VERIFY THAT PROJECT SPECIFIC VALUES OF S_{DS} & z/h RESULT IN SEISMIC FORCES (E_n , E_s) THAT DO NOT EXCEED THE VALUES ON THE DETAILS.

BLDG. SEOR TO VERIFY THE CONCRETE MEETS THE REQUIREMENTS OF THE APPLICABLE ICC-ESR.



MIN. CONC. EDGE DISTANCE

NOTE:
THE FLOOR SEISMIC RESTRAINTS HAVE PASSED TESTING USING AC156 ACCEPTANCE CRITERIA FOR SEISMIC QUALIFICATION BY SHAKE TABLE TESTING OF NON-STRUCTURAL COMPONENTS AND SYSTEMS.



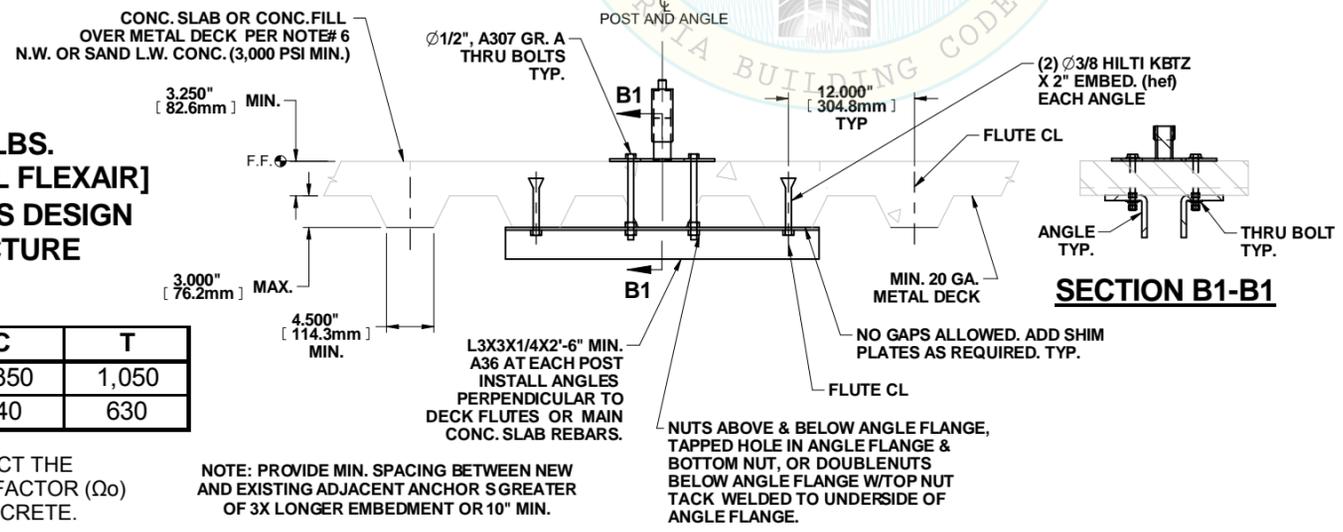
FRONT VIEW

SIDE VIEW

**MAXIMUM OP. WT. = 681 LBS.
[INCLUDING STAND & OPTIONAL FLEXAIR]
MAXIMUM ALLOWABLE STRESS DESIGN
REACTIONS TO THE STRUCTURE
[LBS./POST]**

SLAB LOC'N	$F_p/W_p/1.4$	V	C	T
ELEV. SLAB	1.03	200	1,350	1,050
ON GRADE	0.64	130	940	630

TABULATED REACTIONS DO NOT REFLECT THE APPLICATION OF THE OVERSTRENGTH FACTOR (Ω_0) TO ACCOUNT FOR ANCHORAGE TO CONCRETE.



**SECTION "B-B"
THRU-BOLT ANCHORAGE DETAIL**

NOTE: PROVIDE MIN. SPACING BETWEEN NEW AND EXISTING ADJACENT ANCHOR S GREATER OF 3X LONGER EMBEDMENT OR 10" MIN.

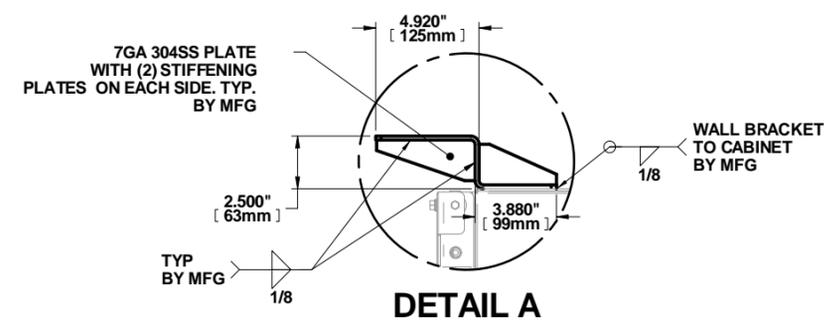
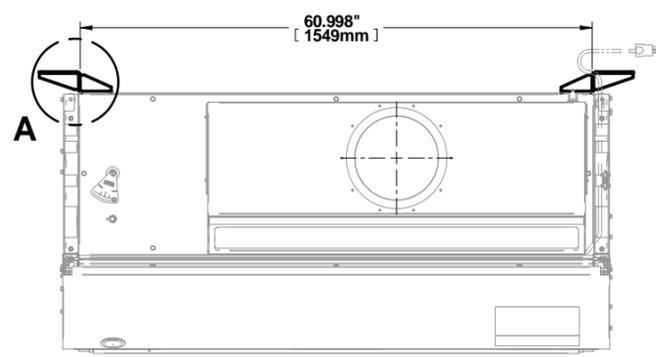


DATE: 04/23/14
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DECIMALS	MACHINED	SHEET METAL	ANGULAR
X	+0.02	+0.06	(P/30) (S/15)
XX	+0.01	+0.03	THIRD ANGLE
XXX	+0.005	+0.015	PROJECTION
FRACTIONS	+1/32		
FINISH	CC	NO. 4	
DATE	10/3/2013	SCALE	
DRAWN	L.McCarthy	SIZE	D
CHECKED		SHEET	2 OF 2
TITLE: STERILIGARD, SG404 CLASS II TYPE A2 FLOOR MOUNTED		OPM SHEET NUMBER: 2 OF 6	
PROJECT: BGA-SG404-A2-1-5-111R		REV: G	

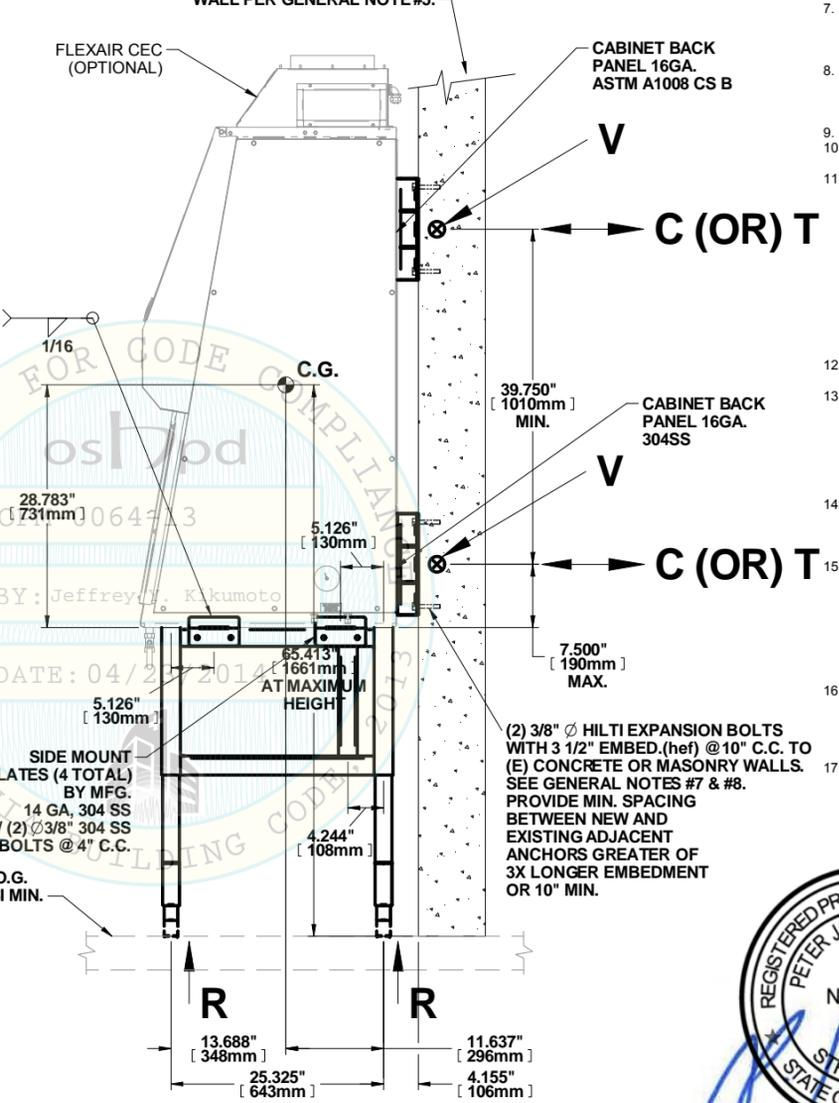
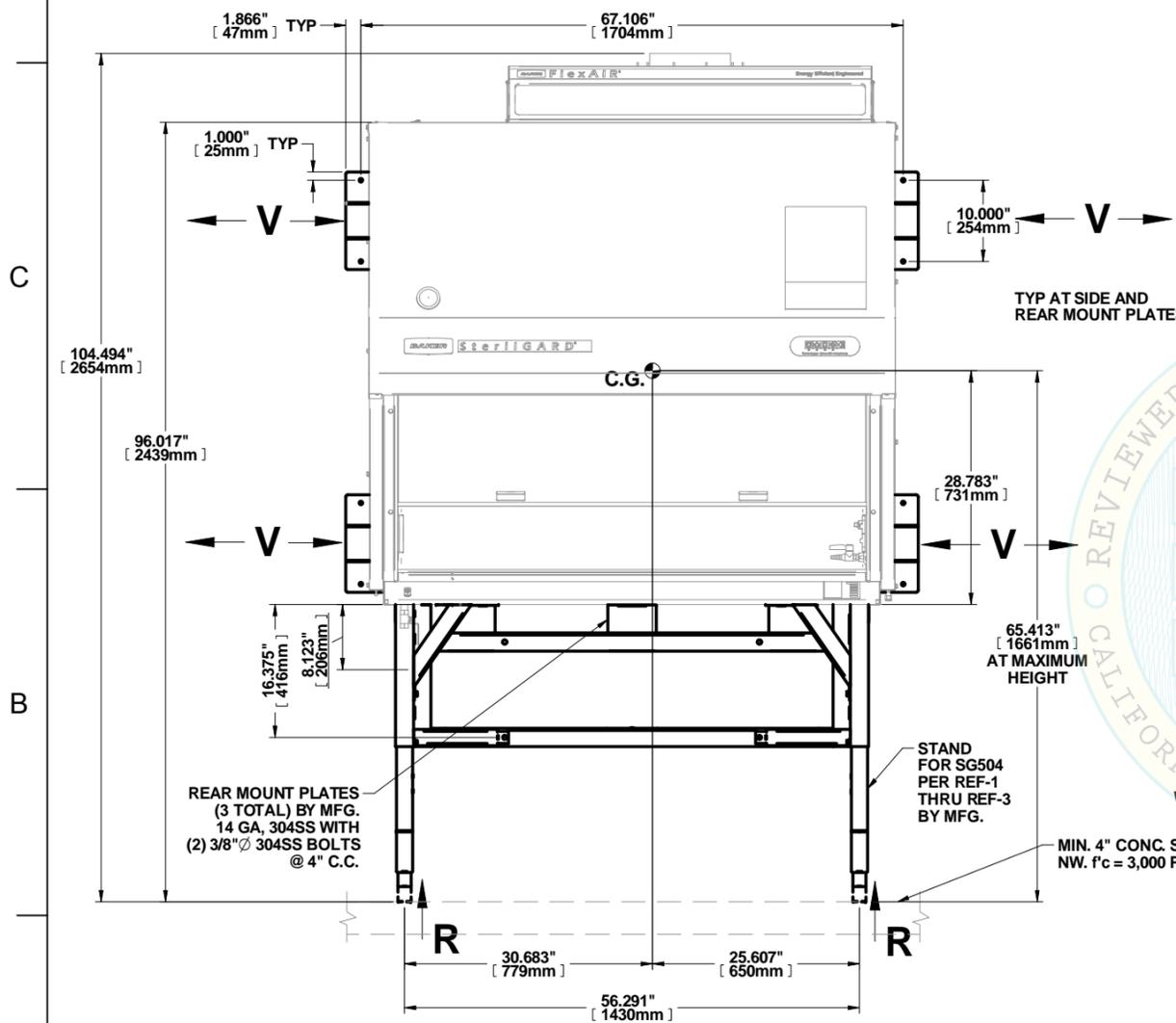
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G	UPDATED DRAWING PER PE COMMENTS	4/22/2014	LAM	-	-



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 - ANCHOR BOLTS INTO (E) GROUT FILLED MASONRY WALL TO BE HILTI KB3 (ICC ESR-1385), CONTINUOUS INSPECTION REQUIRED PER CBC 2013 SECTION 1705A.4. REFER TO ICC ESR REPORT FOR INSTALLATION REQUIREMENTS. TESTING OF EXPANSION ANCHORS PER 2013 CBC, 1913A.7. SEE NOTE# 16 FOR SEOR RESPONSIBILITIES.
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 - CONDITION OF USE REQUIREMENTS IN ACCORDANCE W/ ESR-1385 SECTION 5.0 IS SATISFIED.
 - RESPONSIBILITIES OF THE SEOR OF THE BLDG.:

THE NEW ANCHORS ARE LOCATED AT AN ADEQUATE DISTANCE FROM ANY NEW OR EXISTING ANCHORS. THE SPACING SHOWN IN DETAILS ARE A MINIMUM FOR THE BOLT SIZE SHOWN. THE REQ'D SPACINGS FROM OTHER ANCHORS OF OTHER DIAMETERS & EMBEDMENTS MAY VARY & SHALL BE EVALUATED BY THE SEOR.

BLDG. SEOR TO VERIFY THE CONCRETE MEETS THE REQUIREMENTS OF THE APPLICABLE ICC-ES ESR.



FRONT VIEW
MAXIMUM OP. WT. =815 LBS.
[INCLUDING STAND & OPTIONAL FLEXAIR]
MAXIMUM ALLOWABLE STRESS DESIGN
REACTIONS TO THE STRUCTURE
[LBS./POST]

SLAB LOC'N	Fp/Wp/1.4	V	C	T	R
ELEV. SLAB	1.03	230	240	240	310
ON GRADE	0.64	140	150	150	310

TABULATED REACTIONS DO NOT REFLECT THE APPLICATION OF THE OVERSTRENGTH FACTOR (Ω_0) TO ACCOUNT FOR ANCHORAGE TO CONCRETE.

NOTE:
 THE WALL SEISMIC RESTRAINTS HAVE PASSED TESTING USING AC156 ACCEPTANCE CRITERIA FOR SEISMIC QUALIFICATION BY SHAKE TABLE TESTING OF NON-STRUCTURAL COMPONENTS AND SYSTEMS.

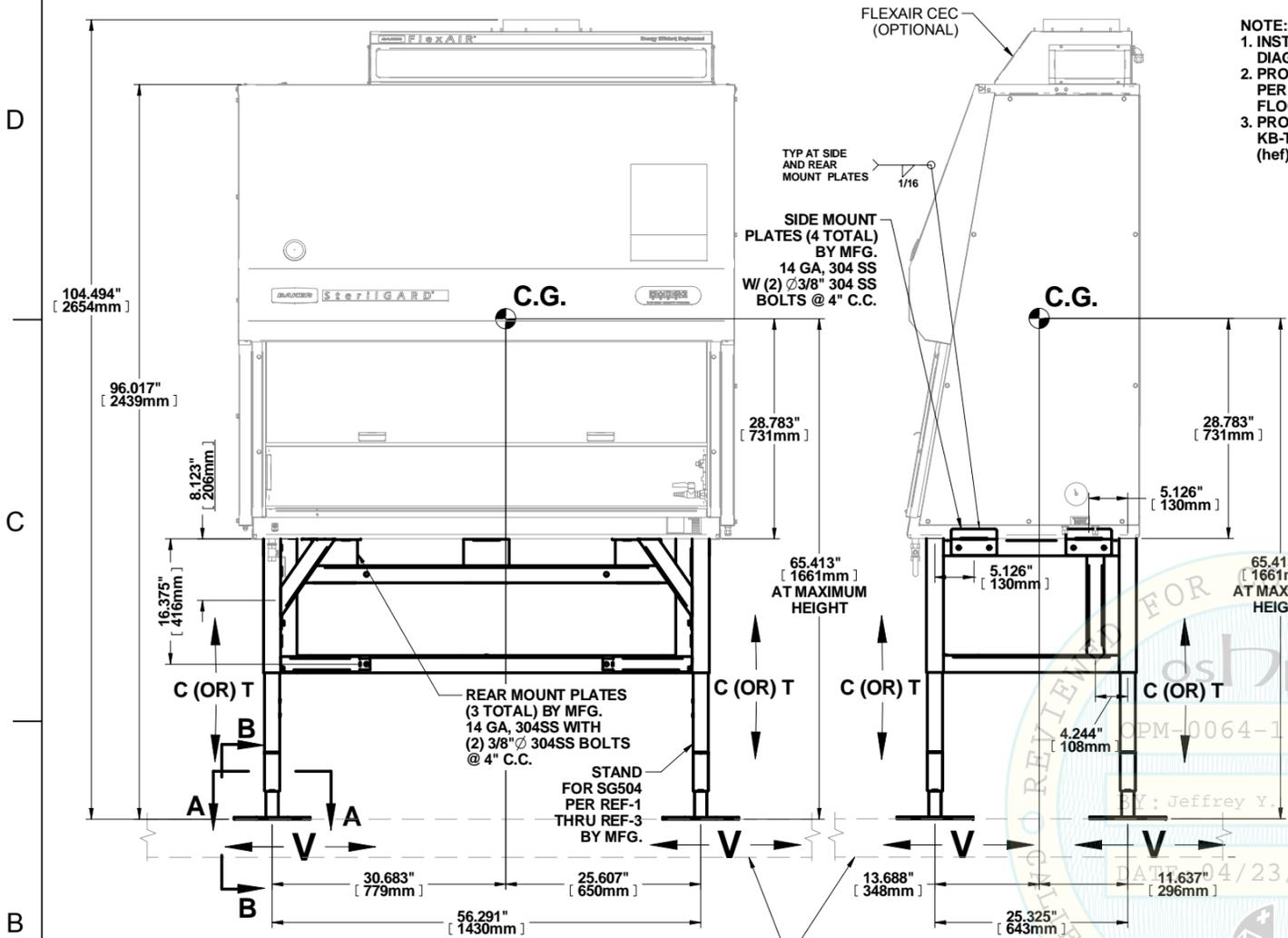


DATE: 04/23/14
 FOR BRANDOW & JOHNSTON, INC

OPM-0064-13

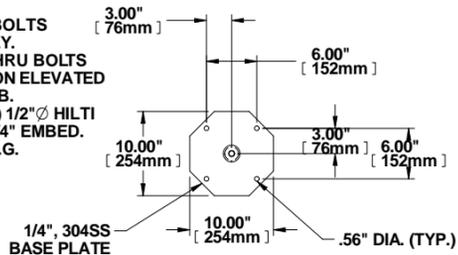
UNLESS OTHERWISE SPECIFIED: DO NOT SCALE DRAWING REMOVE ALL BURRS AND SHARP EDGES U.S. CUSTOMARY TOLERANCES (INCHES):		THIS DRAWING AND ITS CONTENTS ARE THE PROPERTY OF THE BAKER COMPANY. THE INFORMATION CONTAINED HEREON MAY NOT BE USED OR COPIED IN ANY MANNER WITHOUT THE WRITTEN PERMISSION OF THE BAKER COMPANY.	
DECIMALS	FRACTIONS	ANGULAR	OTHER
±0.000	±1/16"	±30'	
±0.005	±1/32"	±15'	
±0.010	±1/64"	±5'	
±0.015	±1/32"	±3'	
±0.020	±1/16"	±1'	
±0.030	±1/8"	±0'	
±0.040	±3/16"	±0'	
±0.050	±1/4"	±0'	
±0.060	±5/16"	±0'	
±0.070	±3/8"	±0'	
±0.080	±7/16"	±0'	
±0.090	±1/2"	±0'	
±0.100	±5/8"	±0'	
±0.125	±3/4"	±0'	
±0.150	±7/8"	±0'	
±0.175	±1"	±0'	
±0.200	±1 1/8"	±0'	
±0.250	±1 1/4"	±0'	
±0.300	±1 1/2"	±0'	
±0.375	±1 3/4"	±0'	
±0.450	±1 7/8"	±0'	
±0.500	±2"	±0'	
±0.625	±2 1/4"	±0'	
±0.750	±2 1/2"	±0'	
±0.875	±2 3/4"	±0'	
±1.000	±3"	±0'	
±1.250	±3 1/2"	±0'	
±1.500	±4"	±0'	
±1.750	±4 1/2"	±0'	
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REV	DESCRIPTION	DATE	DRN	APP-BY	APP-DATE
-	SEE REV. F FOR ALL PAST REVISIONS LISTED	-	-	-	-
G	UPDATED DRAWING PER PE COMMENTS	4/22/2014	LAM	-	-

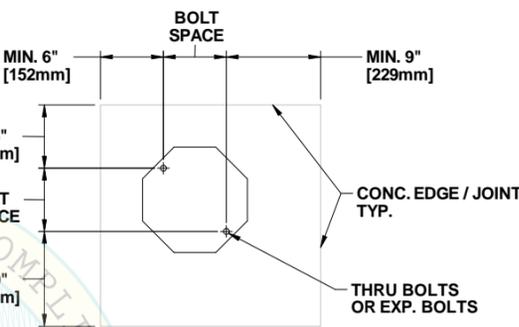


NOTE:

- INSTALL 2 BOLTS DIAGONALLY.
- PROVIDE THRU BOLTS PER "B-B" ON ELEVATED FLOOR SLAB.
- PROVIDE (2) 1/2" HILTI KB-TZ X 3 1/4" EMBED. (hef) AT S.O.G.



SECTION "A-A" BASE PLATE DETAIL



MIN. CONC. EDGE DISTANCE

NOTE:

THE FLOOR SEISMIC RESTRAINTS HAVE PASSED TESTING USING AC156 ACCEPTANCE CRITERIA FOR SEISMIC QUALIFICATION BY SHAKE TABLE TESTING OF NON-STRUCTURAL COMPONENTS AND SYSTEMS.

- GENERAL NOTES:**
- THIS OSHPD PREAPPROVAL OF MANUFACTURER'S CERTIFICATION (OPM) IS BASED ON CBC 2013. THE DEMAND (DESIGN FORCES) FOR USE WITH THIS OPM SHALL BE BASED ON THE CBC 2013.
 - DESIGN SEISMIC FORCES PER ASCE 7-10 SECTION 13.3.1, EQUATIONS 13.3-1, 13.3-2, AND 13.3-3 WHERE $S_{DS} \leq 2.0$, $z/h \leq 1.0$, $a_p = 1.0$, $i_p = 1.5$, AND $R_p = 2.5$. $F_p/W_p/1.4 \leq 1.03$ AND $Q_0 = 2.5$.
 - THIS PRE-APPROVAL IS APPLICABLE TO ANY LOCATION IN THE STATE OF CALIFORNIA AT ANY HEIGHT WITHIN THE BUILDING.
 - THIS PRE-APPROVAL COVERS ONLY THE SUPPORT & ATTACHMENTS OF THE UNIT TO THE HOSPITAL BUILDING'S STRUCTURE.
 - MAX. REACTIONS TO THE STRUCTURE SHOWN ON THE DRAWINGS ARE ALLOWABLE STRESS DESIGN LOADS. SEE NOTES BELOW FOR SEOR'S RESPONSIBILITIES.
 - THE ELEVATED FLOOR SLAB SHALL BE MIN. 5" THICK, 3000 PSI MIN. CONCRETE SOLID SLAB N.W. OR SAND L.W. CONCRETE FILL ABOVE METAL DECK SHALL BE MIN. 3 1/2" THICK, 3,000 PSI MIN. CONCRETE N.W. OR SAND L.W. THE SLAB-ON-GRADE SHALL BE MIN. 6" THICK, N.W. 3,000 PSI MIN.
 - ANCHOR BOLTS INTO (E) CONCRETE TO BE HILTI KB-TZ (ICC ESR-1917). PERIODIC INSPECTION REQUIRED PER CBC 2013 SECTION 1705A.3. REFER TO ICC ESR REPORT FOR INSTALLATION REQUIREMENTS. TESTING OF EXPANSION ANCHORS PER 2013 CBC, 1913A.7 AND NOTE# 11 BELOW.
 - BOLTS THROUGH CONCRETE ON METAL DECK OR ELEVATED SLAB SECTION:
 - BOLTS SHALL BE TORQUED BY 1/4 TURN OF THE NUT AFTER SNUG TIGHT (THE SNUG TIGHT CONDITION IS DEFINED AS THE TIGHTNESS REQUIRED TO BRING THE CONNECTED PILES INTO FIRM CONTACT) CONDITION IS ACHIEVED, UNLESS NOTED OTHERWISE.
 - HOLES FOR THROUGH BOLTS SHALL BE 1/16" LARGER THAN BOLT SIZE (HOLE SIZE = BOLT SIZE + 1/16").
 - THROUGH BOLTS IN CONCRETE SHALL RECEIVE SPECIAL INSPECTION & TESTING (THROUGH BOLTS WITH STEEL-TO-STEEL CONNECTION IN TENSION DO NOT REQUIRE TESTING) IN ACCORDANCE WITH REQUIREMENTS FOR POST-INSTALLED ANCHORS.
 - MAINTAIN CLEARANCE FROM ANY EDGES OF (E) CONCRETE AS SHOWN.
 - LOCATE (E) REINFORCEMENTS BEFORE DRILLING INTO (E) CONCRETE SLAB. DO NOT CUT ANY (E) REINFORCEMENTS.
 - TORQUE TEST:
 - TEST INSTALLED ANCHORS AT A RATE OF 50% IN THE PRESENCE OF INSPECTOR OF RECORD. (I.O.R.) A REPORT OF THE TEST RESULTS SHALL BE SUBMITTED TO OSHPD.
 - ALL WEDGE ANCHORS (LOADED IN EITHER PULLOUT OR SHEAR) SHALL BE TORQUE-TESTED WITH CALIBRATED TORQUE WRENCH TO 25 FT-LBS FOR 3/8" Ø & 40 FT-LBS FOR 1/2" Ø HILTI KB-TZ IN CONCRETE. THE SPECIFIED TORQUE SHALL BE ATTAINED WITHIN 1/4 TURN OF THE NUT. THE SPECIFIED TORQUE IS THE INSTALLATION VERIFICATION TEST LOAD. TORQUED BOLTS SHALL BE TESTED A MINIMUM OF 24 HOURS AFTER INSTALLATION AND SHALL NOT HAVE MORE THAN 1/2 TURN OF THE NUT.
 - WHERE ANCHORS FAIL TESTING, REPLACE DEFICIENT ANCHORS AND RETEST. TEST ALL ANCHORS OF THE SAME TYPE INSTALLED BY THE SAME TRADE AND NOT PREVIOUSLY TESTED.
 - WEDGE ANCHOR EMBEDMENT IS EQUAL TO EFFECTIVE MIN. EMBEDMENT (hef) AS DEFINED IN ICC-ESR REPORTS.
 - SEOR TO VERIFY THAT THE INSTALLATION IS IN CONFORMANCE WITH THE 2013 CBC AND WITH THE NOTES AND DETAILS SHOWN IN THIS PRE-APPROVAL. VERIFY THAT THE EQUIPMENT'S ACTUAL WEIGHT, CG LOCATION, ANCHOR LOCATIONS, ANCHOR DETAILS AND THE MATERIAL AND GAGE OF THE UNIT WHERE ATTACHMENTS ARE MADE CONFORM TO THE INFORMATION SHOWN IN THIS PRE-APPROVAL.
 - THE BUILDING SEOR TO VERIFY THAT THE (N) OR (E) STRUCTURE IS ADEQUATE FOR THE DESIGN LOADS IMPOSED ON IT BY THIS EQUIPMENT IN ADDITION TO ALL OTHER LOADS. PROVIDE AND DESIGN SUPPLEMENTARY FRAMING MEMBERS AS REQ'D. REFER TO MAX. REACTIONS TO THE STRUCTURE.
 - RESPONSIBILITIES OF THE SEOR OR THE BLDG.:
 - THE NEW ANCHORS ARE LOCATED AT AN ADEQUATE DISTANCE FROM ANY NEW OR EXISTING ANCHORS. THE SPACING SHOWN IN DETAILS ARE A MINIMUM FOR THE BOLT SIZE SHOWN. THE REQUIRED SPACINGS FROM OTHER ANCHORS OF OTHER DIAMETERS & EMBEDMENTS MAY VARY & SHALL BE EVALUATED BY THE SEOR. BLDG. SEOR TO VERIFY THAT PROJECT SPECIFIC VALUES OF S_{DS} & z/h RESULT IN SEISMIC FORCES (E_n , E_e) THAT DO NOT EXCEED THE VALUES ON THE DETAILS.
 - BLDG. SEOR TO VERIFY THAT THE CONCRETE MEETS THE REQUIREMENTS OF THE APPLICABLE ICC-ESR.



DATE: 04/23/14
FOR BRANDOW & JOHNSTON, INC

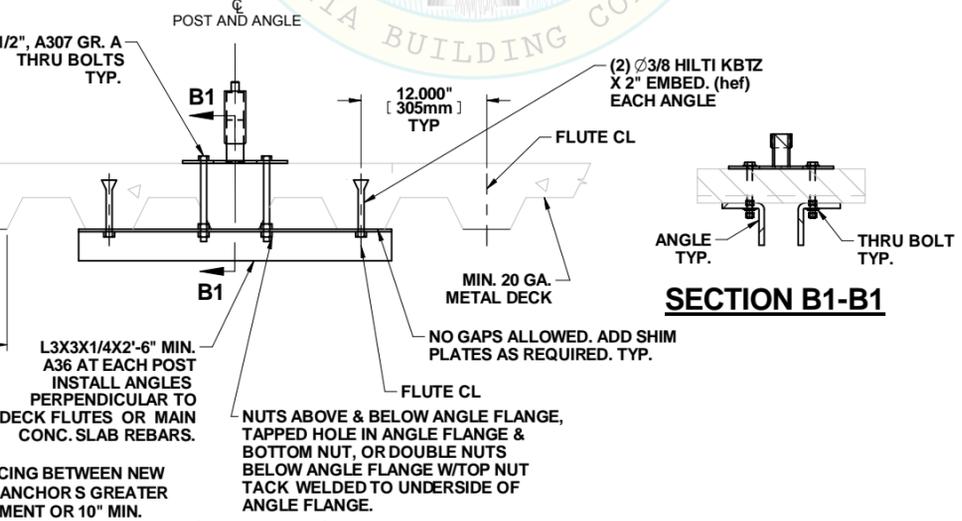
OPM-0064-13

MAXIMUM OP. WT. = 815 LBS.
[INCLUDING STAND & OPTIONAL FLEXAIR]
MAXIMUM ALLOWABLE STRESS DESIGN REACTIONS TO THE STRUCTURE [LBS./POST]

SLAB LOC'N	Fp/Wp/1.4	V	C	T
ELEV. SLAB	1.03	230	1,610	1,220
ON GRADE	0.64	150	1,120	740

TABULATED REACTIONS DO NOT REFLECT THE APPLICATION OF THE OVERSTRENGTH FACTOR (Ω_o) TO ACCOUNT FOR ANCHORAGE TO CONCRETE.

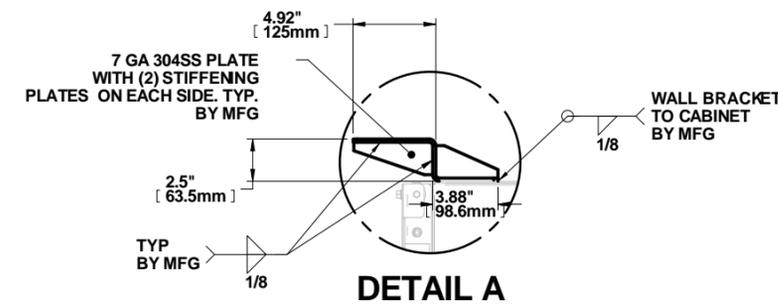
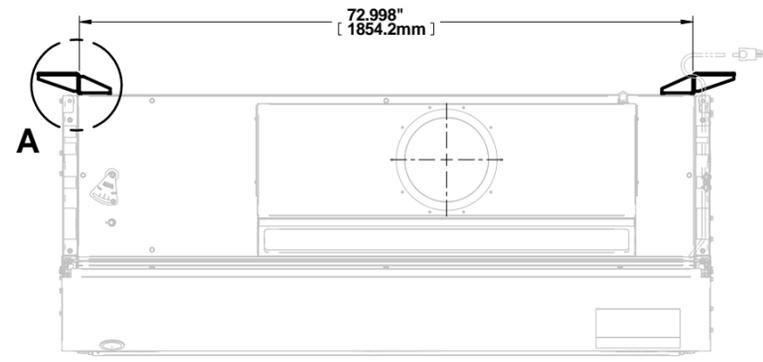
NOTE: PROVIDE MIN. SPACING BETWEEN NEW AND EXISTING ADJACENT ANCHOR S GREATER OF 3X LONGER EMBEDMENT OR 10" MIN.



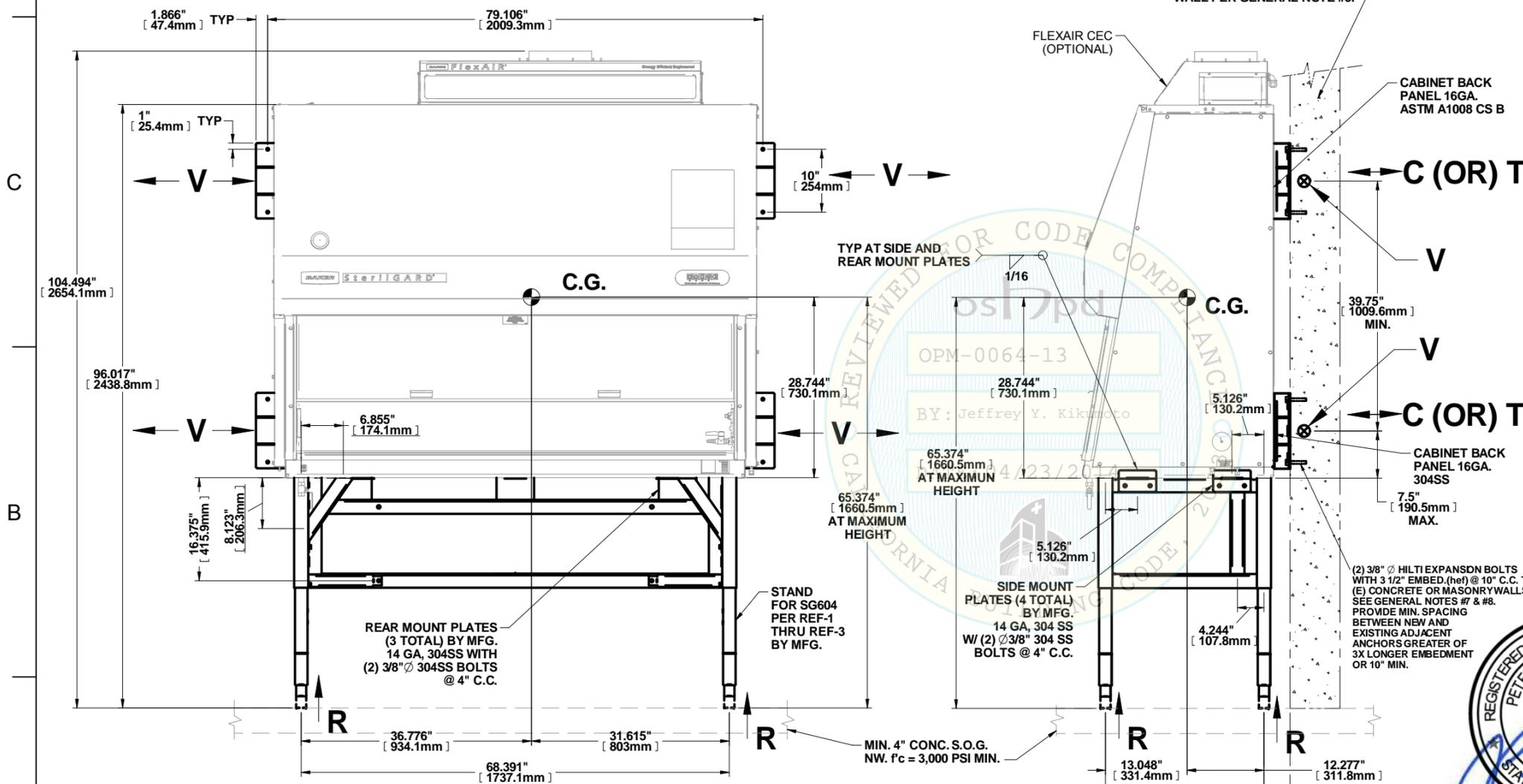
SECTION B1-B1

REVISIONS		TITLE		OPM SHEET NUMBER	
NO.	DATE	DESCRIPTION	BY	DATE	NO.
1	10/4/2013	ISSUED FOR PERMITS	L.McCarthy	10/4/2013	4 OF 6
PROJECT			REV		
L.McCarthy			G		
DATE			SCALE		
10/4/2013			2 OF 2		

REV	DESCRIPTION	DATE	DRN	APP-BY	APP-DATE
-	SEE REV. F FOR ALL PAST REVISIONS LISTED	-	-	-	-
G	UPDATED DRAWING PER PE COMMENTS	4/22/2014	LAM	-	-



- GENERAL NOTES:**
- THIS OSHPD PREAPPROVAL OF MANUFACTURER'S CERTIFICATION (OPM) IS BASED ON CBC 2013. THE DEMAND (DESIGN FORCES) FOR USE WITH THIS OPM SHALL BE BASED ON THE CBC 2013.
 - DESIGN SEISMIC ANCHORAGE FORCES PER ASCE 7-10 SECTION 13.3.1, EQUATIONS 13.3-1, 13.3-2, AND 13.3-3 WHERE $S_{DS} \leq 2.0$, $z/h \leq 1.0$, $a_p = 1.0$, $I_p = 1.5$, AND $R_p = 2.5$. $F_p/W_p/1.4 \leq 1.03$ AND $\Omega_0 = 2.5$.
 - THIS PRE-APPROVAL IS APPLICABLE TO ANY LOCATION IN THE STATE OF CALIFORNIA, AT ANY HEIGHT WITHIN THE BUILDING WHERE $S_{DS} \leq 2.0$.
 - THIS PRE-APPROVAL COVERS ONLY THE SUPPORT & ATTACHMENTS OF THE UNIT TO THE HOSPITAL BUILDING'S STRUCTURE.
 - MAXIMUM REACTIONS TO THE STRUCTURE SHOWN ON THE DRAWINGS ARE ALLOWABLE STRESS DESIGN LOADS. SEE NOTES BELOW FOR SEOR'S RESPONSIBILITIES.
 - THE CONCRETE WALL SHALL BE MIN. 8" THICK $f_c = 3,000$ PSI MIN. CONC. N.W. OR SAND L.W.. THE MASONRY WALL SHALL BE MIN. 8" THICK $f_m = 1,500$ PSI MIN. FULLY GROUTED.
 - ANCHOR BOLTS INTO (E) CONCRETE WALL TO BE HILTI KB-TZ (ICC ESR-1917), PERIODIC INSPECTION REQUIRED PER CBC 2013 SECTION 1705A.3. REFER TO ICC ESR REPORT FOR INSTALLATION REQUIREMENTS. TESTING OF EXPANSION ANCHORS PER 2013 CBC, 1913A.7 AND NOTE# 11 BELOW.
 - ANCHOR BOLTS INTO (E) GROUT FILLED MASONRY WALL TO BE HILTI KB3 (ICC ESR-1385). CONTINUOUS INSPECTION REQUIRED PER CBC 2013 SECTION 1705A.4. REFER TO ICC ESR REPORT FOR INSTALLATION REQUIREMENTS. TESTING OF EXPANSION ANCHORS PER 2013 CBC, 1913A.7. SEE NOTE# 16 FOR SEOR RESPONSIBILITIES.
 - MAINTAIN 12" CLEARANCE FROM ANY EDGES OF (E) CONCRETE OR MASONRY WALL.
 - LOCATE (E) REINFORCEMENTS BEFORE DRILLING INTO (E) CONCRETE OR MASONRY WALL. DO NOT CUT ANY (E) REINFORCEMENTS.
 - TORQUE TEST:
 - TEST INSTALLED ANCHORS AT A RATE OF 50% IN THE PRESENCE OF INSPECTOR OF RECORD. (I.O.R). A REPORT OF THE TEST RESULTS SHALL BE SUBMITTED TO OSHPD.
 - ALL WEDGE ANCHORS (LOADED IN EITHER PULLOUT OR SHEAR) SHALL BE TORQUE-TESTED WITH CALIBRATED TORQUE WRENCH TO 25 FT-LBS FOR HILTI KB-TZ IN CONCRETE & 15 FT-LBS FOR HILTI KB3 IN MASONRY. THE SPECIFIED TORQUE SHALL BE ATTAINED WITHIN 1/2 TURN OF THE NUT. THE SPECIFIED TORQUE IS THE INSTALLATION VERIFICATION TEST LOAD. TORQUED BOLTS SHALL BE TESTED A MINIMUM OF 24 HOURS AFTER INSTALLATION AND SHALL NOT HAVE MORE THAN 1/2 TURN OF THE NUT.
 - WHERE ANCHORS FAIL TESTING, REPLACE DEFICIENT ANCHORS AND RETEST. TEST ALL ANCHORS OF THE SAME TYPE INSTALLED BY THE SAME TRADE AND NOT PREVIOUSLY TESTED.
 - WEDGE ANCHOR EMBEDMENT IS EQUAL TO EFFECTIVE MIN. EMBEDMENT (h_{ef}) AS DEFINED IN ICC-ESR REPORTS.
 - SEOR TO VERIFY THAT THE INSTALLATION IS IN CONFORMANCE WITH THE 2013 CBC AND WITH THE NOTES AND DETAILS SHOWN IN THIS PRE-APPROVAL. VERIFY THAT THE EQUIPMENT'S ACTUAL WEIGHT, CG LOCATION, ANCHOR LOCATIONS, ANCHOR DETAILS AND THE MATERIAL AND GAGE OF THE UNIT WHERE ATTACHMENTS ARE MADE CONFORM TO THE INFORMATION SHOWN IN THIS PRE-APPROVAL. BLDG. SEOR TO VERIFY THAT PROJECT SPECIFIC VALUES OF S_{DS} & z/h RESULT IN SEISMIC FORCES (E_n , E_s) THAT DO NOT EXCEED THE VALUES ON THE DETAILS.
 - THE BUILDING SEOR TO VERIFY THAT THE (N) OR (E) STRUCTURE IS ADEQUATE FOR THE DESIGN LOADS IMPOSED ON IT BY THIS EQUIPMENT IN ADDITION TO ALL OTHER LOADS. PROVIDE & DESIGN SUPPLEMENTARY FRAMING MEMBERS AS REQ'D. REFER TO MAXIMUM REACTIONS TO THE STRUCTURE.
 - SEOR SHALL VERIFY THAT:
 - MASONRY IS NOT CRACKED AS DEFINED IN ICC-ES AC01 SECTION 2.3. SEOR SHALL PROVIDE CALCULATIONS TO SHOW THAT THE MASONRY WALL WOULD NOT CRACK UNDER THE DESIGN EARTHQUAKE LOADS UNDER ALL SERVICE LOAD CONDITIONS; THE WALL HAS TO REMAIN ELASTIC.
 - MASONRY IS FULLY GROUTED IN ACCORDANCE W/ ESR-1385 SECTION 3.2.
 - CONDITION OF USE REQUIREMENTS IN ACCORDANCE W/ ESR-1385 SECTION 5.0 IS SATISFIED.
 - RESPONSIBILITIES OF THE SEOR OF THE BLDG.: THE NEW ANCHORS ARE LOCATED AT AN ADEQUATE DISTANCE FROM ANY NEW OR EXISTING ANCHORS. THE SPACING SHOWN IN DETAILS ARE A MINIMUM FOR THE BOLT SIZE SHOWN. THE REQ'D SPACINGS FROM OTHER ANCHORS OF OTHER DIAMETERS & EMBEDMENTS MAY VARY & SHALL BE EVALUATED BY THE SEOR.
 - BLDG. SEOR TO VERIFY THE CONCRETE MEETS THE REQUIREMENTS OF THE APPLICABLE ICC-ESR.



MAXIMUM OP. WT. = 885 LBS.
[INCLUDING STAND & OPTIONAL FLEXAIR]
MAXIMUM ALLOWABLE STRESS DESIGN
REACTIONS TO THE STRUCTURE
[LBS./POST]

SLAB LOC'N	$F_p/W_p/1.4$	V	C	T	R
ELEV. SLAB	1.03	250	250	250	320
ON GRADE	0.64	160	160	160	320

TABULATED REACTIONS DO NOT REFLECT THE APPLICATION OF THE OVERSTRENGTH FACTOR (Ω_0) TO ACCOUNT FOR ANCHORAGE TO CONCRETE.

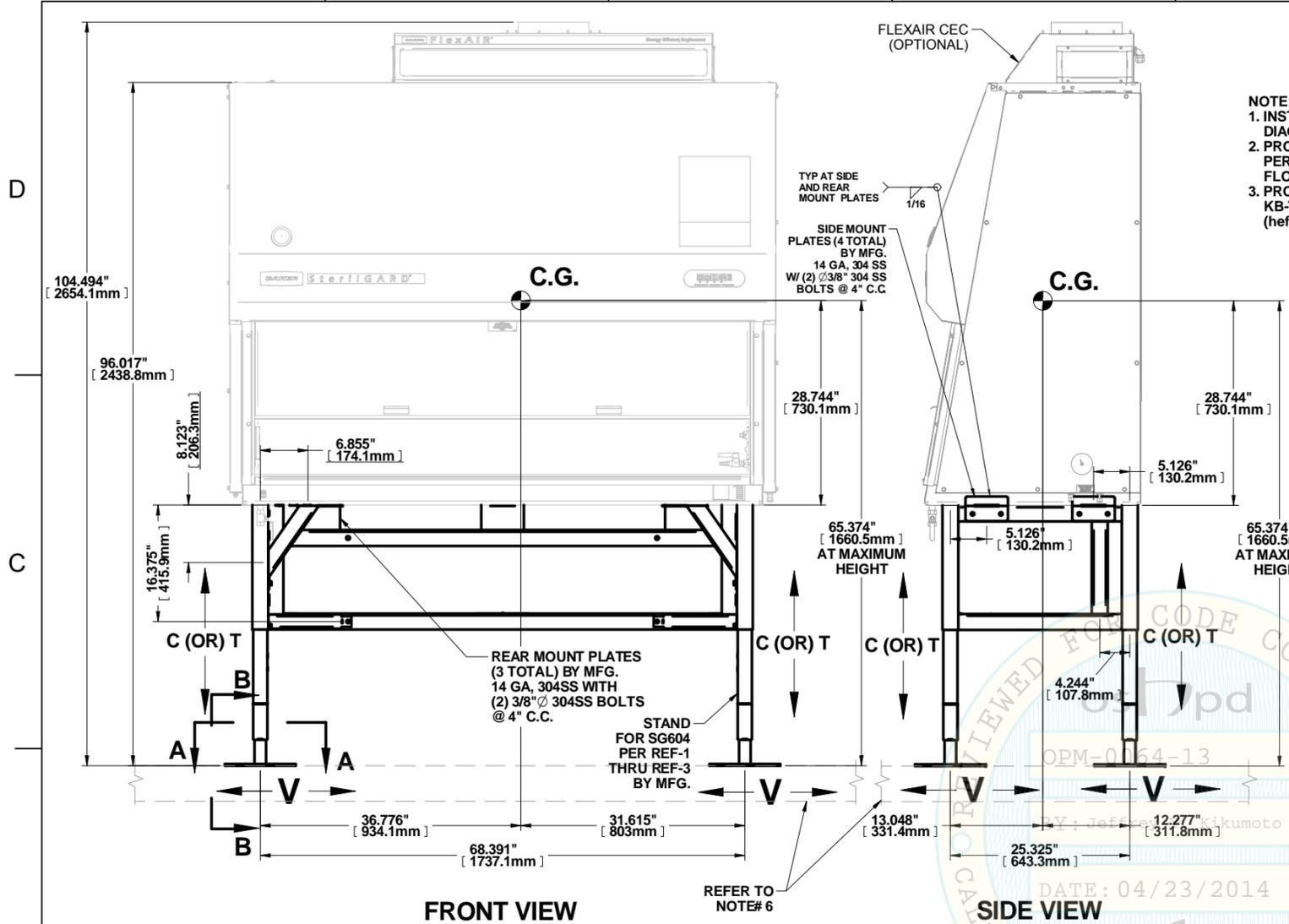


DATE: 04/23/14
FOR BRANDOW & JOHNSTON, INC

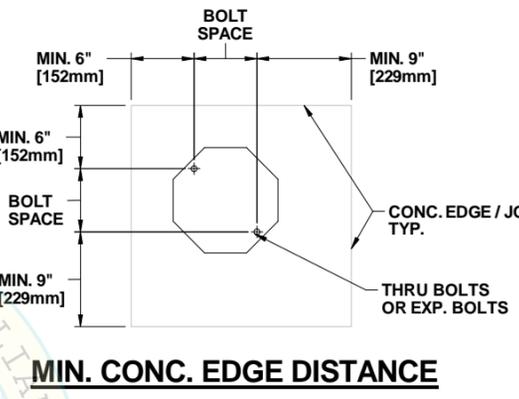
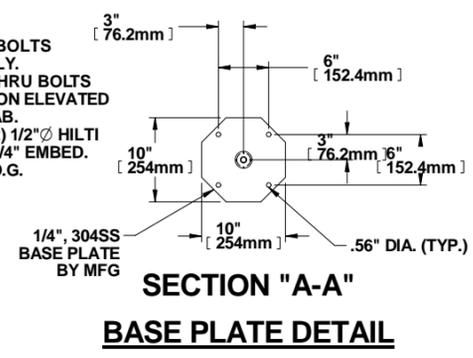
OPM-0064-13

NOTE:
THE WALL SEISMIC RESTRAINTS HAVE PASSED TESTING USING AC156 ACCEPTANCE CRITERIA FOR SEISMIC QUALIFICATION BY SHAKE TABLE TESTING OF NON-STRUCTURAL COMPONENTS AND SYSTEMS.

<small>UNLESS OTHERWISE SPECIFIED: DO NOT SCALE DRAWING REMOVE ALL BURRS AND SHARP EDGES U.S. CUSTOMARY UNITS (INCHES)</small>		<small>THIS DRAWING AND ITS CONTENTS ARE THE PROPERTY OF THE BAKER COMPANY. THE INFORMATION CONTAINED HEREON MAY NOT BE USED OR COPIED IN ANY MANNER WITHOUT THE WRITTEN PERMISSION OF THE BAKER COMPANY.</small>	
<small>DECIMALS</small> X .001 X .002 X .003 X .004 X .005 X .006 X .007 X .008 X .009 X .010 X .011 X .012 X .013 X .014 X .015 X .016 X .017 X .018 X .019 X .020 X .021 X .022 X .023 X .024 X .025 X .026 X .027 X .028 X .029 X .030 X .031 X .032 X .033 X .034 X .035 X .036 X .037 X .038 X .039 X .040 X .041 X .042 X .043 X .044 X .045 X .046 X .047 X .048 X .049 X .050 X .051 X .052 X .053 X .054 X .055 X .056 X .057 X .058 X .059 X .060 X .061 X .062 X .063 X .064 X .065 X .066 X .067 X .068 X .069 X .070 X .071 X .072 X .073 X .074 X .075 X .076 X .077 X .078 X .079 X .080 X .081 X .082 X .083 X .084 X .085 X .086 X .087 X .088 X .089 X .090 X .091 X .092 X .093 X .094 X .095 X .096 X .097 X .098 X .099 X .100 X .101 X .102 X .103 X .104 X .105 X .106 X .107 X .108 X .109 X .110 X .111 X .112 X .113 X .114 X .115 X .116 X .117 X .118 X .119 X .120 X .121 X .122 X .123 X .124 X .125 X .126 X .127 X .128 X .129 X .130 X .131 X .132 X .133 X .134 X .135 X .136 X .137 X .138 X .139 X 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.998 X .999 X .1000	<small>TITLE</small> STERILGARD, MODEL SG604 CLASS II TYPE A2 WALL MOUNTED OPM SHEET NUMBER 5 OF 6		
<small>DATE</small> 9/25/2013 <small>BY</small> LMcCarthy	<small>DATE</small> 9/25/2013 <small>BY</small> LMcCarthy	<small>DATE</small> 9/25/2013 <small>BY</small> LMcCarthy	<small>DATE</small> 9/25/2013 <small>BY</small> LMcCarthy



NOTE:
 1. INSTALL 2 BOLTS DIAGONALLY.
 2. PROVIDE THRU BOLTS PER "B-B" ON ELEVATED FLOOR SLAB.
 3. PROVIDE (2) 1/2" Ø HILTI KB-TZ X 3 1/4" EMBED. (hef) AT S.O.G.



NOTE:
 THE FLOOR SEISMIC RESTRAINTS HAVE PASSED TESTING USING AC156 ACCEPTANCE CRITERIA FOR SEISMIC QUALIFICATION BY SHAKE TABLE TESTING OF NON-STRUCTURAL COMPONENTS AND SYSTEMS.

- GENERAL NOTES:**
- THIS OSHPD PREAPPROVAL OF MANUFACTURER'S CERTIFICATION (OPM) IS BASED ON CBC 2013. THE DEMAND (DESIGN FORCES) FOR USE WITH THIS OPM SHALL BE BASED ON THE CBC 2013.
 - DESIGN SEISMIC FORCES PER ASCE 7-10 SECTION 13.3.1, EQUATIONS 13.3-1, 13.3-2, AND 13.3-3 WHERE $S_{DS} \leq 2.0$, $z/h \leq 1.0$, $a_p = 1.0$, $I_p = 1.5$, AND $R_p = 2.5$. $F_p W_p / 1.4 \leq 1.03$ AND $D_s = 2.5$.
 - THIS PRE-APPROVAL IS APPLICABLE TO ANY LOCATION IN THE STATE OF CALIFORNIA AT ANY HEIGHT WITHIN THE BUILDING.
 - THIS PRE-APPROVAL COVERS ONLY THE SUPPORT & ATTACHMENTS OF THE UNIT TO THE HOSPITAL BUILDING'S STRUCTURE.
 - MAX. REACTIONS TO THE STRUCTURE SHOWN ON THE DRAWINGS ARE ALLOWABLE STRESS DESIGN LOADS. SEE NOTES BELOW FOR SEOR'S RESPONSIBILITIES.
 - THE ELEVATED FLOOR SLAB SHALL BE MIN. 5" THICK, 3000 PSI MIN. CONCRETE SOLID SLAB N.W. OR SAND L.W. CONCRETE FILL ABOVE METAL DECK SHALL BE MIN. 3/4" THICK, 3,000 PSI MIN. CONCRETE N.W. OR SAND L.W. THE SLAB-ON-GRADE SHALL BE MIN. 6" THICK, NV. 3,000 PSI MIN.
 - ANCHOR BOLTS INTO (E) CONCRETE TO BE HILTI KB-TZ (ICC ESR-1917). PERIODIC INSPECTION REQUIRED PER CBC 2013 SECTION 1705A.3. REFER TO ICC ESR REPORT FOR INSTALLATION REQUIREMENTS. TESTING OF EXPANSION ANCHORS PER 2013 CBC, 1913A.7 AND NOTE# 11 BELOW.
 - BOLTS THROUGH CONCRETE ON METAL DECK OR ELEVATED SLAB SECTION:
 - BOLTS SHALL BE TORQUED BY 3/4 TURN OF THE NUT AFTER SNUG TIGHT (THE SNUG TIGHT CONDITION IS DEFINED AS THE TIGHTNESS REQUIRED TO BRING THE CONNECTED PLIES INTO FIRM CONTACT) CONDITION IS ACHIEVED, UNLESS NOTED OTHERWISE.
 - HOLES FOR THROUGH BOLTS SHALL BE 1/16" LARGER THAN BOLT SIZE (HOLE SIZE = BOLT SIZE + 1/16").
 - THROUGH BOLTS IN CONCRETE SHALL RECEIVE SPECIAL INSPECTION & TESTING (THROUGH BOLTS WITH STEEL-TO-STEEL CONNECTION IN TENSION DO NOT REQUIRE TESTING) IN ACCORDANCE WITH REQUIREMENTS FOR POST-INSTALLED ANCHORS.
 - MAINTAIN CLEARANCE FROM ANY EDGES OF (E) CONCRETE AS SHOWN.
 - LOCATE (E) REINFORCEMENTS BEFORE DRILLING INTO (E) CONCRETE SLAB. DO NOT CUT ANY (E) REINFORCEMENTS.
 - TORQUE TEST:
 - TEST INSTALLED ANCHORS AT A RATE OF 50% IN THE PRESENCE OF INSPECTOR OF RECORD. (I.O.R.). A REPORT OF THE TEST RESULTS SHALL BE SUBMITTED TO OSHPD.
 - ALL WEDGE ANCHORS (LOADED IN EITHER PULLOUT OR SHEAR) SHALL BE TORQUE-TESTED WITH CALIBRATED TORQUE WRENCH TO 25 FT-LBS FOR 3/8" Ø & 40 FT-LBS FOR 1/2" Ø HILTI KB-TZ IN CONCRETE. THE SPECIFIED TORQUE SHALL BE ATTAINED WITHIN 1/2 TURN OF THE NUT. THE SPECIFIED TORQUE IS THE INSTALLATION VERIFICATION TEST LOAD. TORQUED BOLTS SHALL BE TESTED A MINIMUM OF 24 HOURS AFTER INSTALLATION AND SHALL NOT HAVE MORE THAN 1/2 TURN OF THE NUT.
 - WHERE ANCHORS FAIL TESTING, REPLACE DEFICIENT ANCHORS AND RETEST. TEST ALL ANCHORS OF THE SAME TYPE INSTALLED BY THE SAME TRADE AND NOT PREVIOUSLY TESTED.
 - WEDGE ANCHOR EMBEDMENT IS EQUAL TO EFFECTIVE MIN. EMBEDMENT (hef) AS DEFINED IN ICC-ESR REPORTS.
 - SEOR TO VERIFY THAT THE INSTALLATION IS IN CONFORMANCE WITH THE 2013 CBC AND WITH THE NOTES AND DETAILS SHOWN IN THIS PRE-APPROVAL. VERIFY THAT THE EQUIPMENT'S ACTUAL WEIGHT, CG LOCATION, ANCHOR LOCATIONS, ANCHOR DETAILS AND THE MATERIAL AND GAGE OF THE UNIT WHERE ATTACHMENTS ARE MADE CONFORM TO THE INFORMATION SHOWN IN THIS PRE-APPROVAL.
 - THE BUILDING SEOR TO VERIFY THAT THE (N) OR (E) STRUCTURE IS ADEQUATE FOR THE DESIGN LOADS IMPOSED ON IT BY THIS EQUIPMENT IN ADDITION TO ALL OTHER LOADS. PROVIDE AND DESIGN SUPPLEMENTARY FRAMING MEMBERS AS REQ'D. REFER TO MAX. REACTIONS TO THE STRUCTURE.
 - RESPONSIBILITIES OF THE SEOR OR THE BLDG.:
 - THE NEW ANCHORS ARE LOCATED AT AN ADEQUATE DISTANCE FROM ANY NEW OR EXISTING ANCHORS. THE SPACING SHOWN IN DETAILS ARE A MINIMUM FOR THE BOLT SIZE SHOWN. THE REQUIRED SPACINGS FROM OTHER ANCHORS OF OTHER DIAMETERS & EMBEDMENTS MAY VARY & SHALL BE EVALUATED BY THE SEOR. BLDG. SEOR TO VERIFY THAT PROJECT SPECIFIC VALUES OF S_{DS} & z/h RESULT IN SEISMIC FORCES (E_n , E_s) THAT DO NOT EXCEED THE VALUES ON THE DETAILS.
 - BLDG. SEOR TO VERIFY THE CONCRETE MEETS THE REQUIREMENTS OF THE APPLICABLE ICC-ES ESR.

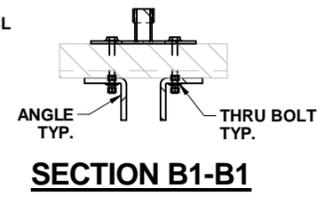
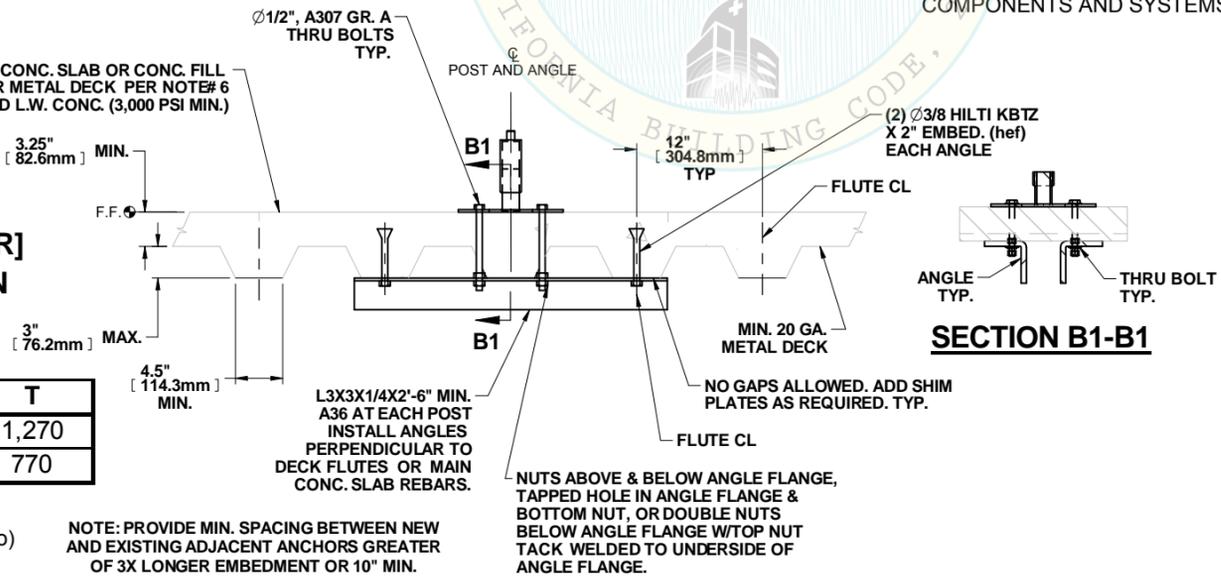
MAXIMUM OP. WT. = 885 LBS.
[INCLUDING STAND & OPTIONAL FLEXAIR]
MAXIMUM ALLOWABLE STRESS DESIGN REACTIONS TO THE STRUCTURE [LBS./POST]

SLAB LOC'N	Fp/Wp/1.4	V	C	T
ELEV. SLAB	1.03	250	1,660	1,270
ON GRADE	0.64	160	1,160	770

TABULATED REACTIONS DO NOT REFLECT THE APPLICATION OF THE OVERSTRENGTH FACTOR (Ω_0) TO ACCOUNT FOR ANCHORAGE TO CONCRETE.

NOTE: PROVIDE MIN. SPACING BETWEEN NEW AND EXISTING ADJACENT ANCHORS GREATER OF 3X LONGER EMBEDMENT OR 10" MIN.

DETAIL "B" THRU-BOLT ANCHORAGE DETAIL

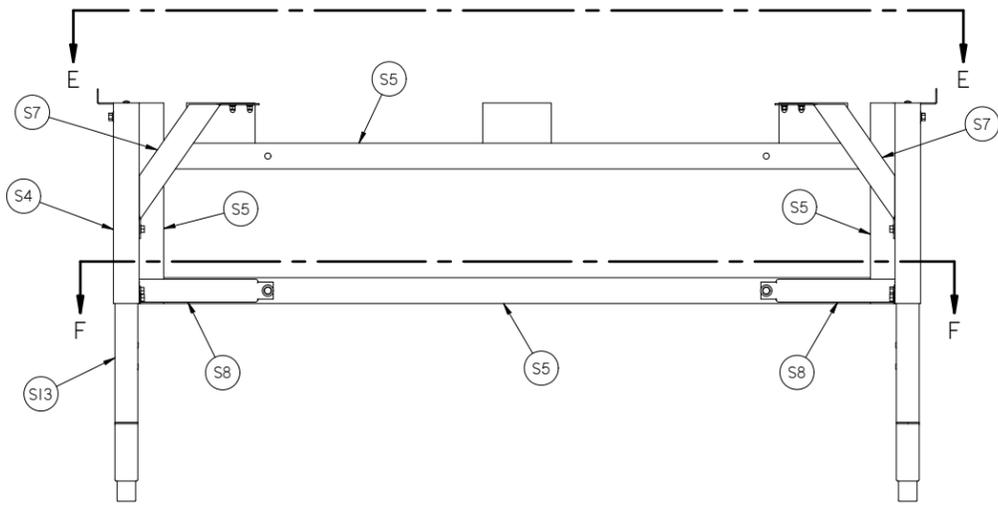


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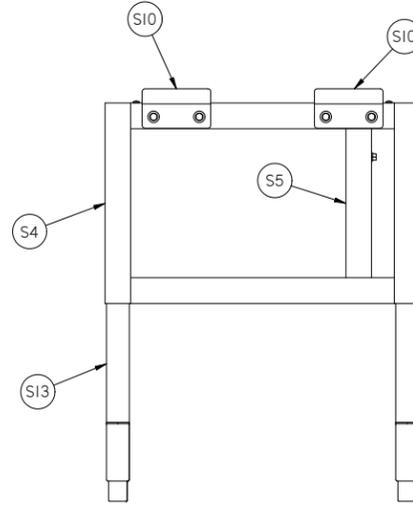
<small>UNLESS OTHERWISE SPECIFIED: DO NOT SCALE DRAWING REMOVE ALL BURRS AND SHARP EDGES USE CUSTOMARY TOLERANCES (INCHES):</small>		<small>THIS DRAWING AND ITS CONTENTS ARE THE PROPERTY OF THE BAKER COMPANY. THE INFORMATION CONTAINED HEREON MAY NOT BE USED OR COPIED IN ANY MANNER WITHOUT THE WRITTEN PERMISSION OF THE BAKER COMPANY.</small>	
<small>DECIMALS</small> 1/16" .0625" 1/8" .125" 1/4" .25" 3/8" .375" 1/2" .5" 5/8" .625" 3/4" .75" 7/8" .875" 1" 1.0" <small>FRACTIONS</small> 1/16" 1/8" 1/4" 3/8" 1/2" 5/8" 3/4" 7/8" <small>FINISH</small> 1/4" R 1/4" F	<small>THIS DRAWING AND ITS CONTENTS ARE THE PROPERTY OF THE BAKER COMPANY. THE INFORMATION CONTAINED HEREON MAY NOT BE USED OR COPIED IN ANY MANNER WITHOUT THE WRITTEN PERMISSION OF THE BAKER COMPANY.</small>	BAKER TITLE: STERILGARD, MODEL SG604 CLASS II TYPE A2 FLOOR MOUNTED OPM SHEET NUMBER: 6 OF 6	SIZE: D DWG NO: BGA-SG604-A2-1-5-111R REV: G
DRAWN: LMcCarthy DATE: 10/4/2013	CHECKED: [] DATE: []	TITLE: [] DATE: []	SHEET: 2 OF 2

REFERENCE ONLY

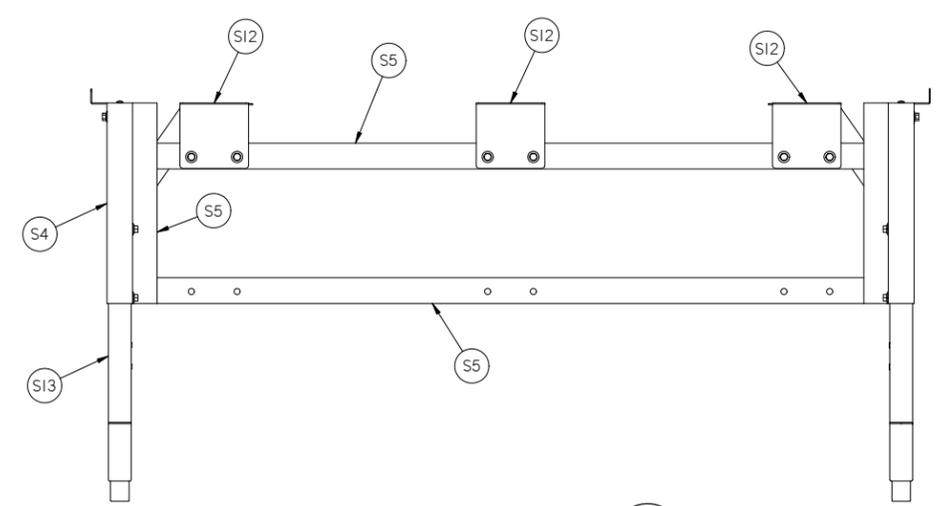
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A	ORIGINAL RELEASE	-	-	-	-
B	ADDED SHEET THREE WITH VIEWS AND GENERAL NOTE	2/18/2014	LAM	-	-
C	UPDATED DRAWING PER PE COMMENTS	3/25/2014	LAM	-	-



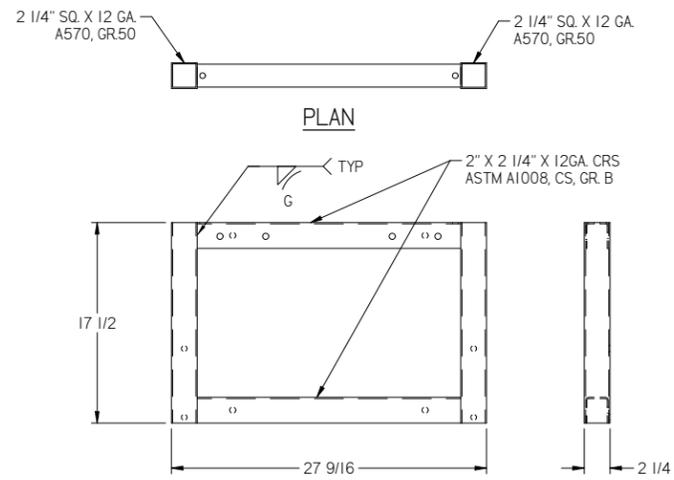
STAND FRONT VIEW **S1**



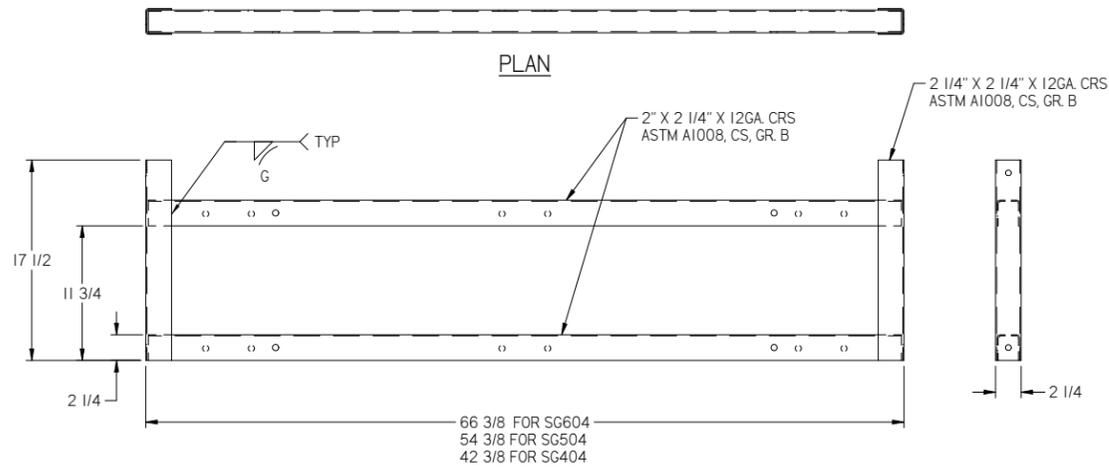
STAND SIDE VIEW **S2**



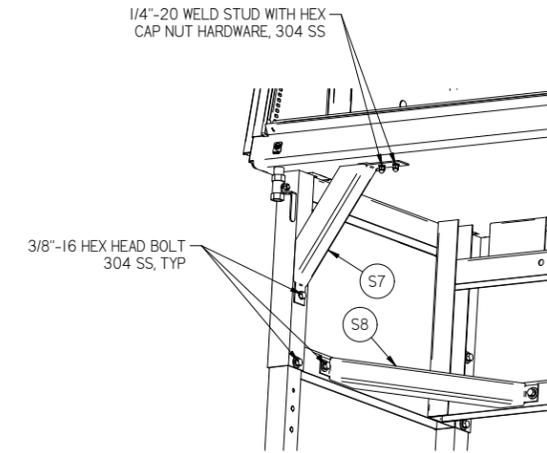
STAND REAR VIEW **S3**



SIDE STAND **S4**

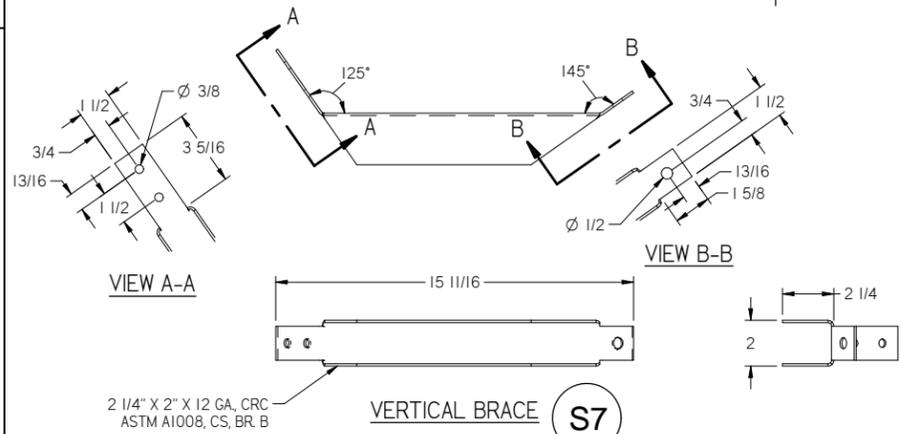


CROSS STAND **S5**

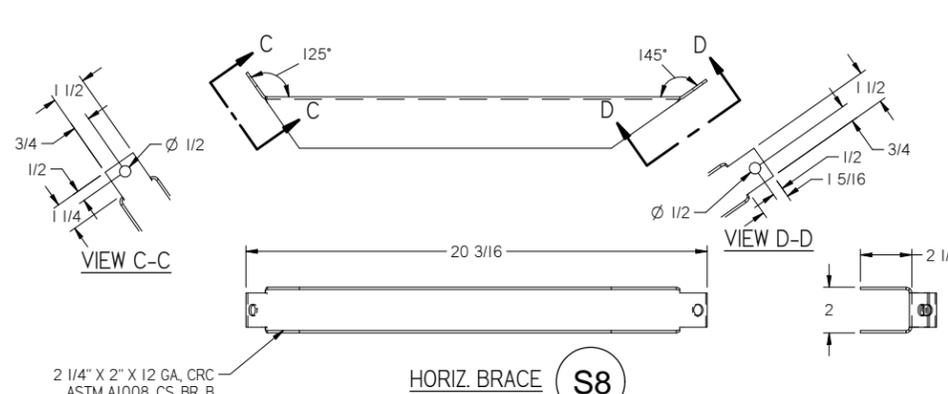


STAND FRAME - BRACING ATTACHMENT **S6**

GENERAL NOTES:
 1. THESE DRAWINGS FOR STERILGARD SEISMIC STAND FOR SG404, SG504, & SG604 ARE NOT FOR CONSTRUCTION BUT FOR INFORMATION ONLY. ALL STAND COMPONENTS, CONNECTION HARDWARE AND WELDS ARE PROVIDED BY THE MANUFACTURER.



VERTICAL BRACE **S7**



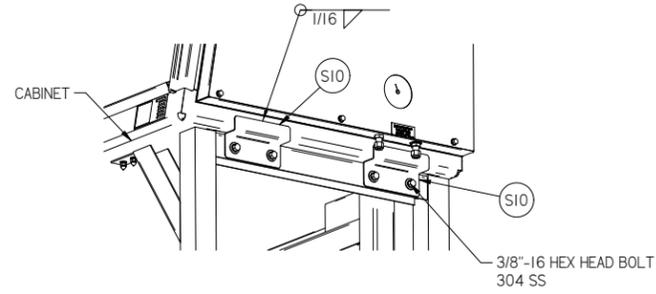
HORIZ. BRACE **S8**

04/23/2014

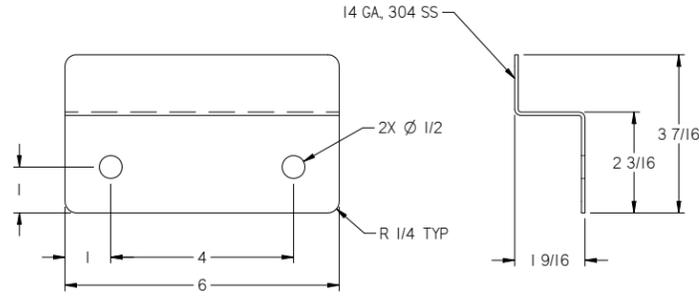
<small>UNLESS OTHERWISE SPECIFIED DO NOT SCALE DRAWING REMOVE ALL BURRS AND SHARP EDGES U.S. CUSTOMARY TOLERANCES: INCHES</small>		<small>THIS DRAWING AND ITS CONTENTS ARE THE PROPERTY OF THE BAKER COMPANY. THE INFORMATION CONTAINED HERE ON MAY NOT BE USED OR COPIED IN ANY MANNER WITHOUT THE WRITTEN PERMISSION OF THE BAKER COMPANY.</small>		THE BAKER COMPANY <small>161 Gatehouse Rd., Sanford, MA 04773 USA "Creating Innovative Assemblies"</small>	
STERILGARD Seismic Stand, Typ Details Stands for SG404, SG504, & SG604		SIZE D	MATERIAL -	DWG NO 365D002	REV C
DESIGNED BY LMcCarthy	DATE 2/12/2014	SCALE -	SQUARE FOOTAGE -	REF-1	Page 9 of 11

REFERENCE ONLY

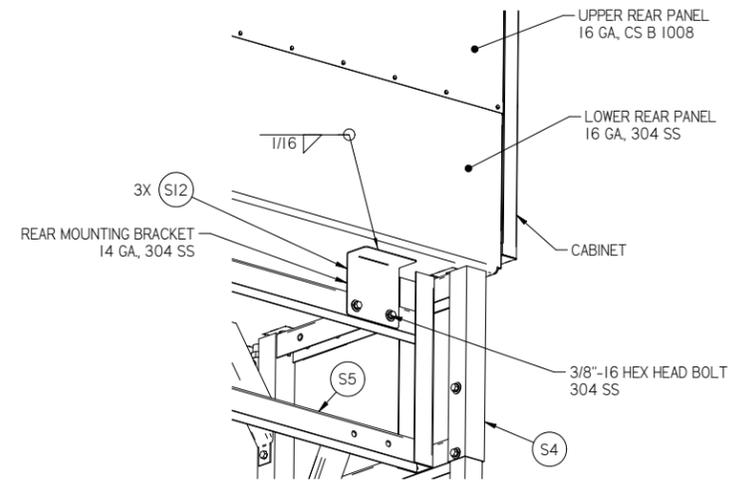
REV	DESCRIPTION	DATE	DRN	APP. BY	APP. DATE
A	ORIGINAL RELEASE	-	-	-	-
B	ADDED SHEET THREE WITH VIEWS AND GENERAL NOTE	2/18/2014	LAM	-	-
C	UPDATED DRAWING PER PE COMMENTS	3/25/2014	LAM	-	-



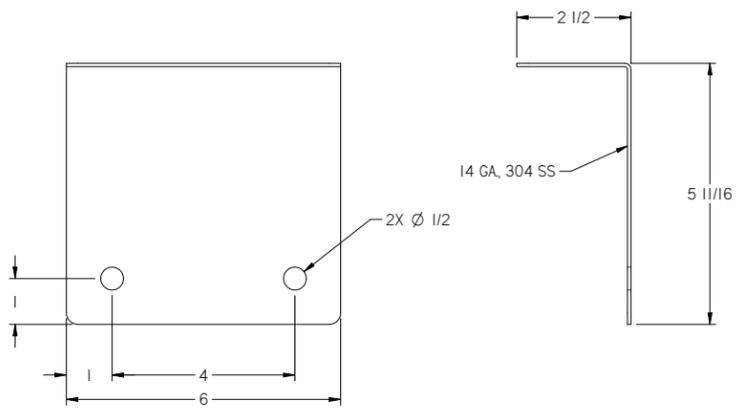
SIDE MOUNTING BRACKET (S9)



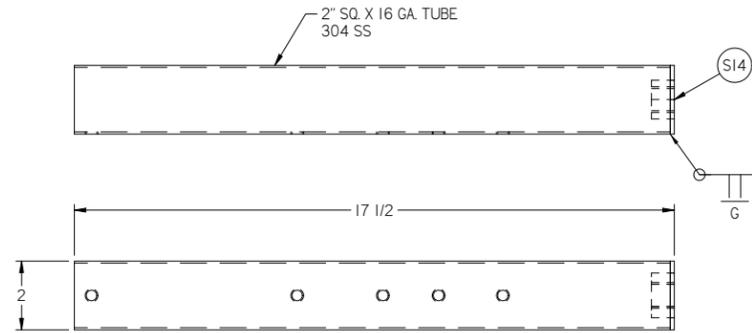
SIDE MOUNTING BRACKET (S10)



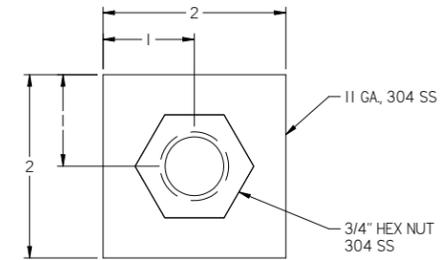
REAR MOUNTING BRACKET ATTACHMENT (S11)



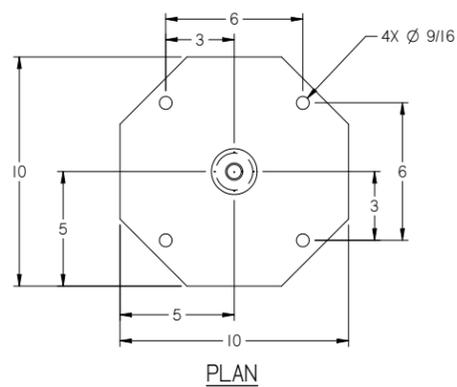
REAR MOUNTING BRACKET (S12)



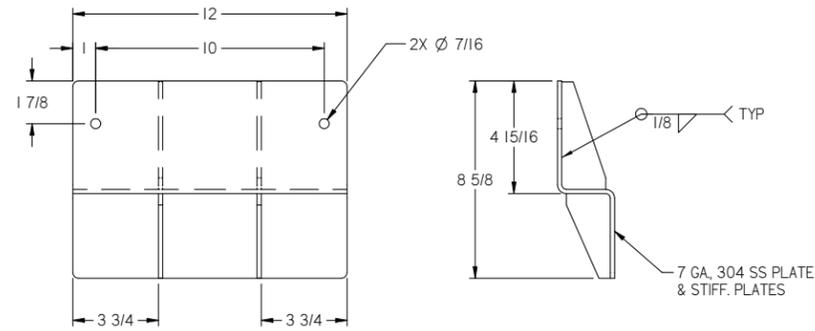
ADJUSTABLE LEG (S13)



ADJUSTABLE LEG END PLATE (S14)



SEISMIC BASE PLATE (S15)

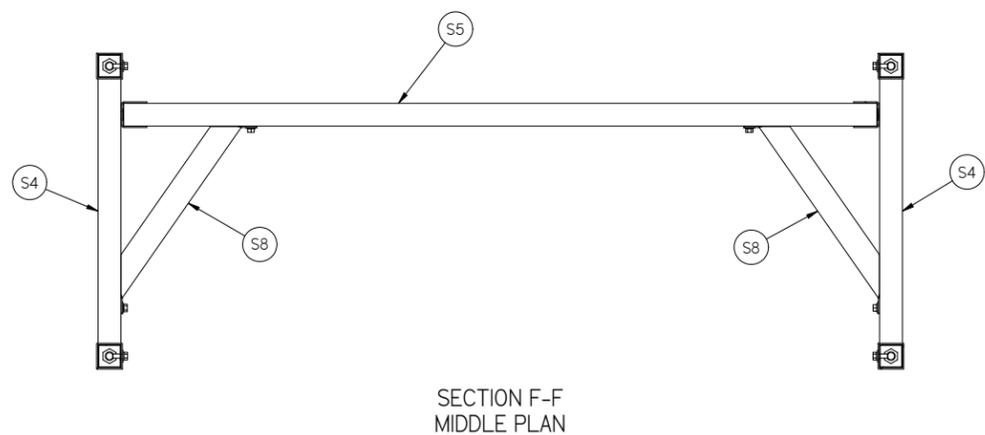
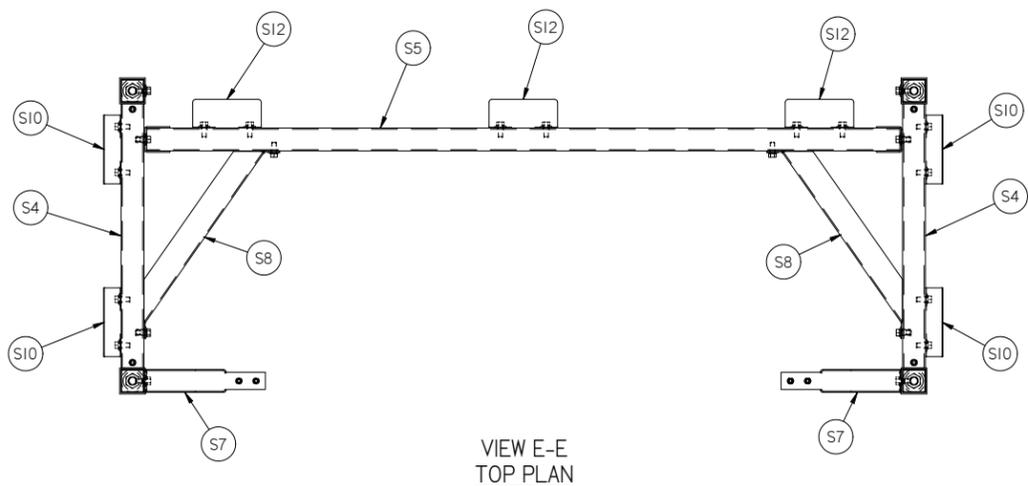


SEISMIC WALL BRACKET (S16)

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TITLE SterilGARD Seismic Stand, Typ Details Stands for SG404, SG504, & SG604		SIZE D	DWG NO 365D002
DESIGNED BY LMcCarthy	DATE 2/12/2014	SCALE -	REV C

REFERENCE ONLY

REV	DESCRIPTION	DATE	DRN	APP. BY	APP. DATE
A	ORIGINAL RELEASE	-	-	-	-
B	ADDED SHEET THREE WITH VIEWS AND GENERAL NOTE	2/18/2014	LAM	-	-
C	UPDATED DRAWING PER PE COMMENTS	3/25/2014	LAM	-	-



04/23/2014

<small>UNLESS OTHERWISE SPECIFIED</small> DO NOT SCALE DRAWING REMOVE ALL BURRS AND SHARP EDGES U.S. CUSTOMARY TOLERANCES: INCHES DECIMALS MACHINED SHEET METAL ANGULAR .X +.00 .005 40:30° (F) .XX +.01 .010 THRU-HOLE .XXX +.02 .015 FRACTIONS 1/32 FINISH ✓ NO. 4 PROJECTION		THIS DRAWING AND ITS CONTENTS ARE THE PROPERTY OF THE BAKER COMPANY. THE INFORMATION CONTAINED HERE ON MAY NOT BE USED OR COPIED IN ANY MANNER WITHOUT THE WRITTEN PERMISSION OF THE BAKER COMPANY.		THE BAKER COMPANY <small>161 Gatehouse Rd., Sanford, Maine 04073 USA "Creating Innovative Assemblies"</small>	
TITLE SterilGARD Seismic Stand, Typ Details Stands for SG404, SG504, & SG604					
DESIGNED BY	DATE	SIZE	MATERIAL	DWG NO.	REV
LAMcCarthy	2/12/2014	D		365D002	C
DETAILS BY	DATE	SCALE	SQUARE FOOTAGE	Page 11 of 11 REF-3	
-	-	-	-		